

TECHNICAL MEMORANDUM



To: Inyo/LA Cooperative Study Team **Date:** April 10, 2003
From: MWH **Reference:** 1341515.030203
Subject: Confining Layer Characteristics Cooperative Study - Aquifer Test Analysis
FINAL

INTRODUCTION

On August 1, 2002, the Los Angeles Department of Water and Power (LADWP) authorized Montgomery Watson Harza (MWH) to continue the Confining Layer Characteristics Cooperative Study with Task 3.2.3, entitled “*Compile, Analyze, and Review Existing Data Sets.*” Subtask 3.2.3.1 of this task is entitled “*Aquifer Test Analysis.*” The scope of this subtask is as follows:

“MWH shall analyze the existing data sets selected in the July 24, 2002 ‘Identification of Methods and Tools for Characterization of the Confining Layer’ Technical Memorandum. LADWP will loan a copy of AQTESOLV for MS Windows software to MWH for analysis. MWH shall utilize the following analysis methods to analyze the nine selected pump test data sets.

- *Hantush-Jacob Method, ‘Classic Leaky Aquifer Solution’*
- *Hantush Method, ‘Modified Leaky Aquifer Solution’*
- *Neuman-Witherspoon Method, ‘Two Aquifer, One Confining Unit Solution’*

Application of three different methods is expected to produce variable values for hydraulic conductivity. As such, MWH shall evaluate the results of this analysis to determine a representative hydraulic conductivity for each given data set. In addition, MWH shall evaluate the results of this analysis with the goal of optimizing and refining future pump testing and associated data collection efforts in the Valley.”

The purpose of this technical memorandum, which represents the deliverable for Subtask 3.2.3.1, is to summarize the results of MWH’s analysis of the selected aquifer test data sets as described in the scope of work.

The technical memorandum is organized into the following sections:

- **Background** – presents a brief description of each of the three methods used to perform the aquifer test analysis.

- **Aquifer Test Analysis Strategy** – presents the general guidelines and assumptions that were utilized to conduct the aquifer test analysis.
- **Aquifer Test Analysis Results** – presents the results of the aquifer test analysis for each pumping well. The wells are organized by wellfield beginning at the northernmost wellfield and continuing south. The nine wells that were analyzed are listed in **Table 1**.
- **Summary of Findings** – presents a summary of aquifer and confining layer parameters estimated during the aquifer test analysis.
- **Recommendations for Future Aquifer Testing** – presents a bulleted list of recommendations to be considered during implementation of future aquifer testing.

Table 1
Summary of Aquifer Tests Evaluated by Wellfield

| Laws | Big Pine | Independence-Oak | Symmes-Shepherd |
|--------------------|--|--------------------|-----------------|
| W387 EM W398 AQ | V014GA V016GA W379 EM W389 EM | W383 EM W384 EM | W395 AQ |

BACKGROUND

As described in more detail in the *Technical Memorandum on the Identification of Methods and Tools for Characterization of the Confining Layer* (MWH, 2002), the following three pump test analysis methods were selected for aquifer test analysis: Hantush-Jacob (1955), Hantush (1960), and Neuman-Witherspoon (1969). A brief description of each of these methods is provided below.

Hantush-Jacob Method (1955)

The Hantush-Jacob Method (1955), from this point on referred to as the Hantush-Jacob Method, is a leaky aquifer analysis method that can be used to yield confined aquifer transmissivity and storativity and confining unit vertical hydraulic conductivity. Analysis is performed by fitting observed displacement vs. time data to a mathematically defined curve. The Hantush-Jacob solution assumes no storage in the confining unit. Originally, the Hantush-Jacob Method did not account for partially penetrating wells; however, the software program utilized during this analysis has modified the solution such that analysis of partially penetrating wells was possible.

Hantush Method (1960)

The Hantush Method (1960), from this point on referred to as the Hantush Method, is a leaky aquifer analysis method that can be used to yield confined aquifer transmissivity and storativity, and the product of confining unit vertical hydraulic conductivity and storativity. Similar to the Hantush-Jacob Method, analysis is performed by fitting observed displacement vs. time data to a mathematically defined curve. This solution has also been modified to account for partially

penetrating wells. Unlike the Hantush-Jacob Method, the Hantush Method does account for storage in the confining unit.

Neuman-Witherspoon Method (1969)

The Neuman-Witherspoon Method (1969), from this point on referred to as the Neuman-Witherspoon Method, is a leaky aquifer analysis method that yields the transmissivity and storativity of the confined aquifer, the vertical hydraulic conductivity and storativity of the confining unit, and the transmissivity and storativity of the unconfined aquifer. Similar to the other two methods described above, analysis is performed by fitting observed displacement vs. time data to a mathematically defined curve. This method accounts for storage in the confining unit and partially penetrating wells. However, unlike the two other methods presented above, this method also accounts for drawdown in the unpumped aquifer. [Note: This method is not to be confused with the Neuman-Witherspoon “ratio” method (1972). In order to perform the “ratio” method, observations of drawdown for wells screened throughout the confining unit must be available. No such measurements were available for any of the existing pump tests available for analysis.]

AQUIFER TEST ANALYSIS STRATEGY

In order for the results of the aquifer test analyses to be both consistent and reproducible, an analysis strategy was created to:

1. Identify data subsets to be analyzed,
2. Make assumptions for data input, and
3. Determine curve matching techniques.

The analysis strategy is outlined below:

1. Identify data subsets to be analyzed:
 - It is difficult to estimate the “distance from the center of pumping” for observation measurements obtained at the pumping well, and at small distances, the analytical solutions are quite sensitive to the distance chosen. As a result, observation well data were selected for analysis over pumping well data wherever possible. However, if adequate drawdown vs. time data were not available for an observation well, pumping well data were analyzed.
 - Observation well data were only used for wells that were screened in the same zone as the pumping well. If no observation wells screened in the same aquifer as the pumped well were utilized, data from the pumping well were selected for analysis.
 - Data collected less than 10 minutes into the aquifer test were not analyzed. This is because the “early”-time data is often suspect because the well is emptying the casing. Thus, the water that is withdrawn from the well casing is not from the aquifer, and less drawdown may be observed than would be predicted by theory. In addition, drawdown measurements were obtained manually. Therefore, from a practical standpoint, the

personnel on the site may have been rushed to take measurements as the water level fell quickly, perhaps resulting in inaccurate measurements.

- The Hantush Method can only be used to analyze early-time data; however, “early-time” is dependent on several aquifer and aquitard parameters that are unknown. For this reason, a qualitative determination was made for each data set as to which data points would be analyzed with the Hantush Method. For the purposes of this analysis, any measurements obtained after an observable change in inflection of the drawdown vs. time curve were discarded from the analysis.

2. Make assumptions for data input:

- Aquifer Data
 - The saturated thickness of the confined aquifer was assumed to be the distance from the bottom of the confining unit to the bottom of the screened interval of the pumped well.
 - A hydraulic conductivity anisotropy ratio (vertical hydraulic conductivity:horizontal hydraulic conductivity) of 1.0 was assumed for the pumped aquifer. Analysis was conducted to determine the sensitivity of the solutions to this assumption. It was determined that for all analyses where the pumping well was fully penetrating as well as for all analyses conducted with the Neuman-Witherspoon Method, changing the hydraulic conductivity anisotropy ratio from 1.0 to 0.1 had no observable effect. Furthermore, it was determined that for wells whose screened intervals perforated at least 80 percent of the saturated aquifer, the effect observed from changing the hydraulic conductivity anisotropy ratio from 1.0 to 0.1 was non-significant. The only pumping well whose screened interval does not penetrate at least 80 percent of the saturated aquifer was well V014GA. For this well, generated solutions for the Hantush-Jacob Method and the Hantush Method could change by up to an order of magnitude if the hydraulic conductivity anisotropy ratio were decreased from 1.0 to 0.1.
- Pumping Well Data
 - If the pumping well was screened over the full saturated aquifer thickness, (as defined above), then the well was assumed to be fully penetrating. Otherwise, the pumping well was assumed to be partially penetrating.
 - A constant pumping rate was assumed for the duration of the aquifer test. (Note: One exception was made to this assumption for well V014GA. This well was pumped for 240 minutes at one pumping rate, and then the pumping rate was doubled for the remainder of the test. This change in pumping rate was modeled during the analysis.)
 - If data from the pumping well were used for analysis, the radial distance chosen was equal to the borehole radius.
- Observation Well Data
 - If the observation well was screened over the full saturated aquifer thickness, (as defined above), then the well was assumed to be fully penetrating. Otherwise, the observation well was assumed to be partially penetrating.

3. Determine curve-matching techniques:

- Initially, AQTESOLV's "automatic curve-matching feature" was utilized.
- If a match was obtained, AQTESOLV's "tweak" feature, which allows the user to make slight adjustments in the generated parameters and observe the effect on the shape of the match curve, was utilized to determine the sensitivity of the curve to variations in each of the generated parameters. Also, the generated aquifer transmissivity and storativity values were compared with existing LADWP estimates (found in the pump test packets) to check for order of magnitude agreement.
- If the curve was sensitive to all generated parameters, no closer visual match could be obtained using the "tweak" feature, and the generated aquifer transmissivity and storativity estimates were within an order of magnitude of existing LADWP estimates, then the match obtained by the automatic curve-matching feature was selected as the "best match".
- If any of the three criteria described in the previous bullet were not met, the parameters were adjusted manually such that the best visual match was obtained, (taking into account parameters generated by prior analyses with other solutions as well as existing LADWP estimates for aquifer transmissivity and storativity). Then, the automatic curve-matching feature was utilized once more to create the "best match".
- At this point, the "tweak" feature was used once more to determine the sensitivity of the curve to the parameters generated. Assuming that the curve was sensitive at least to the parameters necessary to calculate vertical hydraulic conductivity, the analysis was considered complete. The reason that this criterion was applied was to assure that discrete solutions (as opposed to values within a range of possible solutions) were found for the parameters utilized to calculate confining unit properties. The AQTESOLV parameters needed to calculate vertical hydraulic conductivity (or the vertically hydraulic conductivity/storativity product for the Hantush Method) are listed below by method (see **Appendix A** for a glossary of variables and terms):
 - Hantush-Jacob Method: aquifer transmissivity and r/B value
 - Hantush Method: aquifer transmissivity, aquifer storativity, and β_H value
 - Neuman-Witherspoon Method: aquifer transmissivity and r/B value
- If no automatic match could be converged upon, a visual match was created by tweaking the parameters as described above to create a "representative" solution.
- For consistency, it was determined that all parameter solutions generated by AQTESOLV as well as all calculated values based on these parameter solutions would be presented to three significant figures unless otherwise stated.

AQUIFER TEST ANALYSIS RESULTS

The results of the aquifer test analyses are presented herein by wellfield for each of the nine selected aquifer test data sets. This technical memorandum contains the following information:

- A glossary of variables and terms (**Appendix A**),
- Schematic diagrams of the nine aquifer systems analyzed (**Appendix B**),
- Drawdown vs. time data analyzed for each data set (**Appendix C**),

-
- Residual statistics and parameter solutions produced by the AQTESOLV analysis for each data set (**Appendices D – L**),
 - Discussions of the three final solutions obtained for each data set,
 - Two calculated confining unit vertical hydraulic conductivity values for each data set based on the parameter solutions for the Hantush-Jacob Method and the Neuman-Witherspoon Method as well as an estimation of the representative value for each data set. (Note: The confining unit vertical hydraulic conductivity could not be isolated with the Hantush Method, but the confining unit vertical hydraulic conductivity/storativity product is presented.), and
 - A sample calculation for vertical hydraulic conductivity using the Hantush-Jacob Method and the Neuman-Witherspoon Method as well as a sample calculation for the vertical hydraulic conductivity/storativity product using the Hantush Method (**Appendix M**).

Additionally, a summary of the inputs to AQTESOLV that were used during analysis of each of the nine aquifer tests is provided in **Table 2**.

LADWP Well W387 EM (Laws Wellfield)

One observation well, well T734, was monitored in addition to the pumping well during the aquifer test conducted at well W387 EM. However, upon looking at the drawdown vs. time plot for the observation well, it was determined that this well seems to be affected by some outside pumping source and/or external influences, due to significant variation in observed drawdown at late time. For this reason, only the pumping well data from well W387 EM were chosen for analysis, and all of the measurements were analyzed, (except for data collected less than 10 minutes into the test). A summary of the AQTESOLV inputs used to conduct the aquifer test analysis for well W387 EM was presented in **Table 2**, and the drawdown vs. time data that was used for this analysis is located in **Appendix C**. **Table 3** presents a summary of the aquifer test analysis results for well W387 EM. The final solution curves for each analysis method are presented in **Appendix D**.

It is important to note that the top 20 feet of the screened interval for well W387 EM are located in a potential confining/low permeability unit, (fine to medium sand with small gravel, 70 percent brown clay). Because the well was screened in this area, this 20-foot unit of low permeability was considered to be part of the confined aquifer. (AQTESOLV expects the screened interval of a pumping well to begin below the confining unit.) However, if analysis was performed assuming the 20-foot low permeability unit to be part of the confining unit and assuming the screened interval to start immediately below this unit, different results might be obtained.

Table 2
Summary of AQTESOLV Inputs for Aquifer Test Analysis

| | | Laws | | Big Pine | | | | Independence-Oak | | Symmes-Shepherd |
|--|--|---|---|---|--|--|--|--|---|---|
| | | Well W387 EM | Well W398 AQ | Well V014GA | Well V016GA | Well W379 EM | Well W389 EM | Well W383 EM | Well W384 EM | Well W395 AQ |
| AQUIFER CHARACTERISTIC INPUTS | Saturated Aquifer Thickness | 370 feet | 367 feet | 65 feet | 48 feet | 200 feet | 200 feet | 355 feet | 340 feet | 462 feet |
| | Kr/Kz Ratio | 1:1 | 1:1 | 1:1 | 1:1 | 1:1 | 1:1 | 1:1 | 1:1 | 1:1 |
| | Confining Unit Thickness ¹ (see Appendix B – Schematic Diagrams) | 50 feet | 109 feet | 190 feet | 140 feet | 100 feet | 110 feet | 150 feet | 50 feet | 30 feet |
| PUMPING WELL INPUTS | Fully vs. Partially Penetrating Well | Partially | Partially | Partially | Partially | Fully | Fully | Partially | Fully | Partially |
| | Distance from Base of Confining Unit to Top of Perforated Interval | 40 feet | 17 feet | 25 feet | 8 feet | 0 feet | 0 feet | 54 feet | 0 feet | 32 feet |
| | Distance from Base of Confining Unit to Bottom of Perforated Interval | 370 feet | 367 feet | 65 feet | 48 feet | 200 feet | 200 feet | 355 feet | 340 feet | 462 feet |
| | Pumping Rate | 482.6 ft ³ /min (3610 gpm) | 401.0 ft ³ /min (3000 gpm) | 12.7 ft ³ /min (95 pgm) for time ≤ 240 min 25.4 ft ³ /min (190 gpm) for time > 240 min | 34.6 ft ³ /min (259 gpm) | 409.1 ft ³ /min (3060 gpm) | 418.4 ft ³ /min (3130 gpm) | 297.0 ft ³ /min (2222 gpm) | 126.9 ft ³ /min (949 gpm) | 407.7 ft ³ /min (3050 gpm) |
| INPUTS FOR WELL SELECTED FOR DATA ANALYSIS | Well Name | W387 EM | W398 AQ | V014GA | V016GA | T627 | T736 | T632 | T633 | W395 AQ |
| | Radial Distance from Pumping Well | 1.17 feet (based on 28" borehole diameter) | 1.17 feet (based on 28" borehole diameter) | 0.667 feet (based on 16" borehole diameter) | 0.625 feet (based on 15" borehole diameter) | 57 feet | 48.5 feet | 81.5 feet | 88.5 feet | 1.17 feet (based on 28" borehole diameter) |
| | Fully vs. Partially Penetrating Well | Partially | Partially | Partially | Partially | Partially | Partially | Partially | Partially | Partially |
| | Distance from Base of Confining Unit to Top of Perforated Interval | 40 feet | 17 feet | 25 feet | 8 feet | 100 feet | 100 feet | 160 feet | 200 feet | 32 feet |
| | Distance from Base of Confining Unit to Bottom of Perforated Interval | 370 feet | 367 feet | 65 feet | 48 feet | 150 feet | 150 feet | 200 feet | 240 feet | 462 feet |
| | Number of Time vs. Drawdown Measurements Analyzed | 42 | 32 | 40 | 17 | 25 | 45 | 47 | 51 | 65 |

¹The confining unit thickness is not an input used during AQTESOLV analysis. However, it is provided in this table as it is needed to calculate vertical hydraulic conductivity values from the parameters yielded by AQTESOLV analysis.

Table 3
Summary of Aquifer Test Analysis Results for Well W387 EM

| Analysis Results | Aquifer Test Analysis Method | | |
|--|------------------------------|-----------------------|---|
| | Hantush-Jacob (1955) | Hantush (1960) | Neuman-Witherspoon (1969) |
| Residual Statistics¹ | | | |
| Mean | -0.0001757 | -0.00002489 | -0.002161 |
| Variance | 0.2422 | 0.2699 | 0.2456 |
| Standard Error | 0.4921 | 0.5196 | 0.4956 |
| Parameter Solutions² | | | |
| T [gpd/ft] | 57,100 | 56,800 | 53,600 |
| S [unitless] | 4.25×10^{-3} | 3.75×10^{-3} | 2.73×10^{-3} |
| r/B [unitless] | 1.34×10^{-3} | Not Applicable | 1.34×10^{-3} |
| β_H [unitless] | Not Applicable | 2.97×10^{-4} | Not Applicable |
| β_{NW} [unitless] | Not Applicable | Not Applicable | 3.30×10^{-4} |
| T^U [gpd/ft] | Not Applicable | Not Applicable | 1.34×10^{-1} |
| S^U [unitless] | Not Applicable | Not Applicable | 1.00×10^0 |

¹ Residual statistics presented for the Hantush Method should not be compared with values from the other two methods as this analysis was performed only on a subset of the data used for analysis with the other two methods.

² Bolded parameters can be varied over orders of magnitude without affecting the curve shape.

Hantush-Jacob (1955) Analysis

Analysis with the Hantush-Jacob Method produced a solution curve that provides a very good visual match with the pumping well data. Calculated transmissivity and storativity values agree with earlier estimates by the LADWP (LADWP, 1993). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity.

Hantush (1960) Analysis

Analysis with the Hantush Method was conducted only on the first 420 minutes of data, (excluding the first 10 minutes). This analysis produced a solution curve that provides a good visual match with the pumping well data, and calculated transmissivity and storativity values agree well with earlier LADWP estimates (LADWP, 1993). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity/storativity product.

Neuman-Witherspoon (1969) Analysis

Analysis with the Neuman-Witherspoon Method produced a solution curve that provides a very good visual match with the pumping well data. Calculated confined aquifer transmissivity and storativity values agree with earlier LADWP estimates (LADWP, 1993). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic

conductivity. (However, it should be noted that the calculated unconfined aquifer transmissivity and storativity values can be varied over many orders of magnitude with no visual effect on the curve shape.)

Confining Unit Vertical Hydraulic Conductivity Near Well W387 EM

Calculated vertical hydraulic conductivity values for the confining unit present at well W387 EM are presented in **Table 4**. (Note: A sample calculation of these values is presented in **Appendix M**.) From this table, it is observed that the confining unit vertical hydraulic conductivity values calculated from the Hantush-Jacob solution and the Neuman-Witherspoon solution are virtually identical. The representative confining unit vertical hydraulic conductivity near well W387 EM is estimated to be an average of the two solutions, or 3.64 gpd/ft².

Table 4
Estimation of the Confining Unit Vertical Hydraulic Conductivity Near Well W387 EM

| Analysis Method | Confining Unit Vertical Hydraulic Conductivity [gpd/ft ²] | Confining Unit Vertical Hydraulic Conductivity/Storativity Product [gpd/ft ²] |
|-----------------------------|---|---|
| Hantush-Jacob (1955) | 3.73 | Not Applicable |
| Hantush (1960) ¹ | Not Applicable | 0.0110 |
| Neuman-Witherspoon (1969) | 3.54 | Not Applicable |
| Representative Value | 3.64 | Not Applicable |

¹ For the Hantush Method (1960), only the product of confining unit vertical hydraulic conductivity and confining unit storativity can be calculated.

LADWP Well W398 AQ (Laws Wellfield)

The pumping well data were selected for analysis of the aquifer test conducted at well W398 AQ. No observation wells screened in the same aquifer as well W398 AQ were monitored during this test. All of the pumping well data were analyzed, (except for data collected less than 10 minutes into the test). A summary of the AQTESOLV inputs used to conduct the aquifer test analysis for well W398 AQ was presented in **Table 2**, and the drawdown vs. time data that was used for this analysis is located in **Appendix C**. **Table 5** presents a summary of the aquifer test analysis results for well W398 AQ. The final solution curves for each analysis method are presented in **Appendix E**.

Hantush-Jacob (1955) Analysis

Automatic curve matching analysis of the data from well W398 AQ with the Hantush-Jacob Method was not successful. AQTESOLV was unable to converge on a reasonable solution. This failure is attributed to the low quality of the data. The data imply that there were numerous changes in pumping rate. In order to generate some estimate of the aquifer properties near well W398 AQ, a visual match was obtained by setting the transmissivity and storativity near earlier

estimates by the LADWP (LADWP, 1991). Next, the parameters were tweaked as little as possible to obtain a reasonable visual match. The results of this match are presented in **Table 5**. However, other solutions are possible, and these values should be used with caution as they were estimated based on visual inspection alone.

Table 5
Summary of Aquifer Test Analysis Results for Well W398 AQ

| Analysis Results | Aquifer Test Analysis Method | | |
|--|------------------------------|-----------------------|---|
| | Hantush-Jacob (1955) | Hantush (1960) | Neuman-Witherspoon (1969) |
| Residual Statistics¹ | | | |
| Mean | Not Applicable | Not Applicable | Not Applicable |
| Variance | Not Applicable | Not Applicable | Not Applicable |
| Standard Error | Not Applicable | Not Applicable | Not Applicable |
| Parameter Solutions² | | | |
| T [gpd/ft] | 81,600 | 74,400 | 71,100 |
| S [unitless] | 5.76×10^{-4} | 1.02×10^{-3} | 1.41×10^{-3} |
| r/B [unitless] | 3.72×10^{-4} | Not Applicable | 8.32×10^{-4} |
| β_H [unitless] | Not Applicable | 4.57×10^{-4} | Not Applicable |
| β_{NW} [unitless] | Not Applicable | Not Applicable | 3.98×10^{-4} |
| T^U [gpd/ft] | Not Applicable | Not Applicable | $1.08 \times 10^{+3}$ |
| S^U [unitless] | Not Applicable | Not Applicable | 1.00×10^{-1} |

¹ No residual information is available as AQTESOLV could not converge on a solution for this data set.

² All solutions presented in this table are estimated only and should be used with caution.

³ Bolded parameters can be varied over orders of magnitude without affecting the curve shape.

Hantush (1960) Analysis

Similar to the Hantush-Jacob Method, automatic curve-matching analysis with the Hantush Method was unsuccessful. Again AQTESOLV could not converge on a solution; therefore, an estimated solution was obtained by creating a reasonable visual match as described above. The results of this match are presented in **Table 5**, but again, these results are not unique and should be used with caution.

Neuman-Witherspoon (1969) Analysis

As with the other two methods, automatic curve-matching analysis with the Neuman-Witherspoon Method was unsuccessful. Again AQTESOLV could not converge on a solution, so an estimated solution was obtained by creating a reasonable visual match as described above. The results of this match are presented in **Table 5**, but again, these results are not unique and should be used with caution. (Note also that the calculated unconfined aquifer transmissivity can be varied over many orders of magnitude with no visual effect on the curve shape.)

Confining Unit Vertical Hydraulic Conductivity Near Well W398 AQ

Calculated vertical hydraulic conductivity values for the confining unit present at well W398 AQ are presented in **Table 6**. A comparison between the confining unit vertical hydraulic conductivity values calculated by the Hantush-Jacob Method and the Neuman-Witherspoon Method reveals that the two values are within an order of magnitude of each other. However, these values are based on highly questionable parameter solutions, and with the low quality of available data, there is no reason to rely in the results generated by either method. For this reason, the representative confining unit vertical hydraulic conductivity near well W398 AQ is estimated to be an average of the two solutions, or 2.41 gpd/ft².

Table 6
Estimation of the Confining Unit Vertical Hydraulic Conductivity Near Well W398 AQ

| Analysis Method | Confining Unit Vertical Hydraulic Conductivity [gpd/ft ²] | Confining Unit Vertical Hydraulic Conductivity/Storativity Product [gpd/ft ²] |
|-----------------------------|---|---|
| Hantush-Jacob (1955) | 0.897 | Not Applicable |
| Hantush (1960) ¹ | Not Applicable | 0.0202 |
| Neuman-Witherspoon (1969) | 3.92 | Not Applicable |
| Representative Value | 2.41 | Not Applicable |

¹ For the Hantush Method (1960), only the product of confining unit vertical hydraulic conductivity and confining unit storativity can be calculated.

U.S. Geological Survey Well V014GA (Big Pine Wellfield)

The pumping well data were selected for analysis of the aquifer test conducted at well V014GA. No observation wells screened in the same aquifer as well V014GA were monitored during this test. All of the pumping well data were analyzed, (except for data collected less than 10 minutes into the test and less than 10 minutes after the change in pumping rate at 240 minutes). A summary of the AQTESOLV inputs used to conduct the aquifer test analysis for well V014GA was presented in **Table 2**, and the drawdown vs. time data that was used for this analysis is located in **Appendix C**. **Table 7** presents a summary of the aquifer test analysis results for well V014GA. The final solution curves for each analysis method are presented in **Appendix F**.

Hantush-Jacob (1955) Analysis

Analysis with the Hantush-Jacob Method produced a solution curve that provides a relatively good visual match with the pumping well data collected at well V014GA. However, the curve does not fit the data obtained before the increase in pumping rate. The calculated transmissivity value agrees well with earlier estimates by the LADWP (LADWP, 1985). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity.

Table 7
Summary of Aquifer Test Analysis Results for Well V014GA

| Analysis Results | Aquifer Test Analysis Method | | |
|--|------------------------------|-----------------------|--------------------------------------|
| | Hantush-Jacob (1955) | Hantush (1960) | Neuman-Witherspoon (1969) |
| Residual Statistics¹ | | | |
| Mean | -0.1378 | -0.0002322 | -0.0358 |
| Variance | 0.0315 | 0.0004548 | 0.01982 |
| Standard Error | 0.1775 | 0.02133 | 0.1408 |
| Parameter Solutions² | | | |
| T [gpd/ft] | 40,600 | 36,200 | 32,600 |
| S [unitless] | 3.41×10^{-3} | 2.75×10^{-2} | 1.41×10^{-3} |
| r/B [unitless] | 4.56×10^{-4} | Not Applicable | 4.96×10^{-4} |
| β_H [unitless] | Not Applicable | 5.16×10^{-4} | Not Applicable |
| β_{NW} [unitless] | Not Applicable | Not Applicable | 1.11×10^{-4} |
| T ^U [gpd/ft] | Not Applicable | Not Applicable | 1.13×10^0 |
| S ^U [unitless] | Not Applicable | Not Applicable | 1.58×10^{-2} |

¹ Residual statistics presented for the Hantush Method should not be compared with values from the other two methods as this analysis was performed only on a subset of the data used for analysis with the other two methods.

² Bolded parameters can be varied over orders of magnitude without affecting the curve shape.

Hantush (1960) Analysis

Analysis with the Hantush Method was conducted only on the first 240 minutes of data, (excluding the first 10 minutes). This analysis produced a solution curve that provides a relatively good visual match with the pumping well data, and the calculated transmissivity value agrees well with earlier LADWP estimates (LADWP, 1985). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity/storativity product.

Neuman-Witherspoon (1969) Analysis

Analysis with the Neuman-Witherspoon Method produced a solution curve that provides a relatively good match with the pumping well data. However, the curve does not match the data obtained immediately after the increase in pumping rate. Calculated confined aquifer transmissivity agrees well with earlier LADWP estimates (LADWP, 1985). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity. (However, it should be noted that the calculated unconfined aquifer transmissivity can be varied over many orders of magnitude with no visual effect on the curve shape.)

Confining Unit Vertical Hydraulic Conductivity Near Well V014GA

Calculated vertical hydraulic conductivity values for the confining unit present at well V014GA are presented in **Table 8**. From this table, it is observed that the confining unit vertical hydraulic conductivity values calculated from the Hantush-Jacob solution and the Neuman-Witherspoon

solution are virtually identical. The representative confining unit vertical hydraulic conductivity near well V014GA is therefore estimated as the average of the two values, or 3.52 gpd/ft².

Table 8
Estimation of the Confining Unit Vertical Hydraulic Conductivity Near Well V014GA

| Analysis Method | Confining Unit Vertical Hydraulic Conductivity [gpd/ft ²] | Confining Unit Vertical Hydraulic Conductivity/Storativity Product [gpd/ft ²] |
|-----------------------------|---|---|
| Hantush-Jacob (1955) | 3.60 | Not Applicable |
| Hantush (1960) ¹ | Not Applicable | 1.81 |
| Neuman-Witherspoon (1969) | 3.43 | Not Applicable |
| Representative Value | 3.52 | Not Applicable |

¹ For the Hantush Method (1960), only the product of confining unit vertical hydraulic conductivity and confining unit storativity can be calculated.

U.S. Geological Survey Well V016GA (Big Pine Wellfield)

The pumping well data were selected for analysis of the aquifer test conducted at well V016GA. No observation wells screened in the same aquifer as well V016GA were monitored during this test. Only early-time pumping well data were analyzed, (excluding data collected less than 10 minutes into the test), as changes in the pumping rate at time greater than 150 minutes adversely affected the quality of the data. A summary of the AQTESOLV inputs used to conduct the aquifer test analysis for well V016GA was presented in **Table 2**, and the drawdown vs. time data that was used for this analysis is located in **Appendix C**. **Table 9** presents a summary of the aquifer test analysis results for well V016GA. The final solution curves for each analysis method are presented in **Appendix G**.

Hantush-Jacob (1955) Analysis

Automatic curve-matching analysis of the data from well V016GA with the Hantush-Jacob Method was not successful. AQTESOLV was unable to converge on a reasonable solution. This failure is attributed to the fact that only 150 minutes of acceptable data exist for this well. In order to generate some estimate of the aquifer properties near well V016GA, a visual match was obtained by setting the transmissivity near the earlier estimates by the LADWP, (LADWP, 1985), and by setting the storativity close to the value obtained at well V014GA (which is located very close to well V016GA). Then, the parameters were tweaked as little as possible to obtain a reasonable visual match. The results of this match are presented in **Table 9**. However, other solutions are possible, and these values should be used with caution as they were estimated based on visual inspection alone.

Table 9
Summary of Aquifer Test Analysis Results for Well V016GA

| Analysis Results | Aquifer Test Analysis Method | | |
|--|------------------------------|-----------------------|---|
| | Hantush-Jacob (1955) | Hantush (1960) | Neuman-Witherspoon (1969) |
| Residual Statistics¹ | | | |
| Mean | Not Applicable | Not Applicable | Not Applicable |
| Variance | Not Applicable | Not Applicable | Not Applicable |
| Standard Error | Not Applicable | Not Applicable | Not Applicable |
| Parameter Solutions^{2,3} | | | |
| T [gpd/ft] | 25,700 | 23,800 | 19,900 |
| S [unitless] | 3.16×10^{-4} | 5.65×10^{-4} | 1.59×10^{-3} |
| r/B [unitless] | 7.41×10^{-4} | Not Applicable | 2.69×10^{-3} |
| β_H [unitless] | Not Applicable | 4.80×10^{-4} | Not Applicable |
| β_{NW} [unitless] | Not Applicable | Not Applicable | 4.18×10^{-4} |
| T ^U [gpd/ft] | Not Applicable | Not Applicable | 1.08×10^{-3} |
| S ^U [unitless] | Not Applicable | Not Applicable | 8.91×10^{-2} |

¹ No residual information is available as AQTESOLV could not converge on a solution for this data set.

² All solutions presented in this table are estimated only and should be used with caution.

³ Bolded parameters can be varied over orders of magnitude without affecting the curve shape.

Hantush (1960) Analysis

Similar to the Hantush-Jacob Method, automatic curve-matching analysis with the Hantush Method was unsuccessful. Again AQTESOLV could not converge on a solution; therefore, an estimated solution was obtained by creating a reasonable visual match as described above. The results of this match are presented in **Table 9**, but again, these results are not unique and should be used with caution.

Neuman-Witherspoon (1969) Analysis

As with the other two methods, automatic curve-matching analysis with the Neuman-Witherspoon Method was unsuccessful. Again AQTESOLV could not converge on a solution; therefore, an estimated solution was obtained by creating a reasonable visual match as described above. The results of this match are presented in **Table 9**, but again, these results are not unique and should be used with caution. (Note also that the calculated unconfined aquifer transmissivity can be varied over many orders of magnitude with no visual effect on the curve shape.)

Confining Unit Vertical Hydraulic Conductivity Near Well V016GA

Calculated vertical hydraulic conductivity values for the confining unit present at well V016GA are presented in **Table 10**. A comparison between the confining unit vertical hydraulic conductivity values calculated by the Hantush-Jacob Method and the Neuman-Witherspoon Method reveals that the latter value is approximately one order of magnitude higher than the

former value. However, these values are based on highly questionable parameter solutions, and with the limited data available, there is no reason to favor the results of either method. For this reason, the representative confining unit vertical hydraulic conductivity near well V016GA is estimated to be an average of the two solutions, or 24.4 gpd/ft².

Table 10
Estimation of the Confining Unit Vertical Hydraulic Conductivity Near Well V016GA

| Analysis Method | Confining Unit Vertical Hydraulic Conductivity [gpd/ft ²] | Confining Unit Vertical Hydraulic Conductivity/Storativity Product [gpd/ft ²] |
|-----------------------------|---|---|
| Hantush-Jacob (1955) | 4.33 | Not Applicable |
| Hantush (1960) ¹ | Not Applicable | 0.0152 |
| Neuman-Witherspoon (1969) | 44.4 | Not Applicable |
| Representative Value | 24.4 | Not Applicable |

¹ For the Hantush Method (1960), only the product of confining unit vertical hydraulic conductivity and confining unit storativity can be calculated.

LADWP Well W379 EM (Big Pine Wellfield)

Two observation wells, well T627 and well W378 EM, were monitored in addition to the pumping well during the aquifer test conducted at well W379 EM. However, well W378 EM is located almost 700 feet from well W379 EM whereas well T627 is located only 57 feet from the pumping well. Because of the distance of well W378 EM from the pumping well, there is an increased likelihood that this well could be influenced by external factors other than the pumping of well W379 EM. For this reason, observation measurements from well T627 only were chosen for analysis. All of the observation well data from well T627 were analyzed, (except for data collected less than 10 minutes into the test). A summary of the AQTESOLV inputs used to conduct the aquifer test analysis for well W379 EM was presented in **Table 2**, and the drawdown vs. time data that was used for this analysis is located in **Appendix C**. **Table 11** presents a summary of the aquifer test analysis results for well W379 EM. The final solution curves for each analysis method are presented in **Appendix H**.

Hantush-Jacob (1955) Analysis

Analysis with the Hantush-Jacob Method produced a solution curve that provides a good visual match with the observation data collected at well T627. Calculated transmissivity and storativity values agree well with earlier estimates by the LADWP (LADWP, 1992). The curve shape is sensitive to aquifer transmissivity; however, not enough late-time data exist to accurately estimate the r/B value. The value converged upon by AQTESOLV, 1×10^{-5} , can be manually increased to approximately 5×10^{-3} without affecting the early-time shape of the Hantush-Jacob curve for which data exist. For this reason, the latter r/B value was used to calculate the upper limit of the vertical hydraulic conductivity for this system.

Table 11
Summary of Aquifer Test Analysis Results for Well W379 EM

| Analysis Results | Aquifer Test Analysis Method | | |
|--|------------------------------|-----------------------|---|
| | Hantush-Jacob (1955) | Hantush (1960) | Neuman-Witherspoon (1969) |
| Residual Statistics | | | |
| Mean | 0.005037 | 0.0004869 | 0.002433 |
| Variance | 0.1087 | 0.1088 | 0.126 |
| Standard Error | 0.3297 | 0.3299 | 0.355 |
| Parameter Solutions^{1,2} | | | |
| T [gpd/ft] | 90,300 | 90,200 | 90,200 |
| S [unitless] | 1.38×10^{-3} | 1.38×10^{-3} | 1.38×10^{-3} |
| r/B [unitless] | 5.00×10^{-3} | Not Applicable | 3.00×10^{-3} |
| β_H [unitless] | Not Applicable | 5.00×10^{-4} | Not Applicable |
| β_{NW} [unitless] | Not Applicable | Not Applicable | 1.00×10^{-5} |
| T ^U [gpd/ft] | Not Applicable | Not Applicable | 6.93×10^{-7} |
| S ^U [unitless] | Not Applicable | Not Applicable | 1.00×10^0 |

¹ Italicized values represent the upper limit of a range of possible values for a given parameter.

² Bolded parameters can be varied over orders of magnitude without affecting the curve shape.

Hantush (1960) Analysis

Analysis with the Hantush Method was conducted on all of the observation data and produced a solution curve that provides a good visual match with the observation well data. Calculated transmissivity and storativity values agree well with earlier LADWP estimates (LADWP, 1992). The curve shape is sensitive to aquifer transmissivity and storativity; however, the data collected were not sufficient to identify a unique value for β_H . The value converged upon, 1×10^{-5} , can be increased to approximately 5×10^{-4} without affecting the early-time shape of the Hantush curve for which data exist. For this reason, the latter β_H value was used to calculate the upper limit of the confining unit vertical hydraulic conductivity/storativity product for this system.

Neuman-Witherspoon (1969) Analysis

Analysis with the Neuman-Witherspoon Method produced a solution curve provides a good visual match with the available observation data collected at well T627. Again, calculated confined aquifer transmissivity and storativity values agree well with earlier LADWP estimates (LADWP, 1992). The curve shape is sensitive to confined aquifer transmissivity; however, as with the Hantush-Jacob analysis, not enough late-time data exist to accurately estimate the r/B value. The value converged upon by AQTESOLV, 1×10^{-5} , can be manually increased to approximately 3×10^{-3} without affecting the early-time shape of the Neuman-Witherspoon curve for which data exist. For this reason, the latter r/B value was used to calculate the upper limit of the vertical hydraulic conductivity for this system. (Note that the calculated β_{NW} , unconfined aquifer transmissivity, and unconfined aquifer storativity values can be varied over many orders of magnitude with no visual effect on the curve shape.)

Confining Unit Vertical Hydraulic Conductivity Near Well W379 EM

Calculated upper limits for vertical hydraulic conductivity for the confining unit present at well W379 EM are presented in **Table 12**. From this table, it is observed that the confining unit vertical hydraulic conductivity value calculated from the Hantush-Jacob solution is approximately three times greater than the value calculated from the Neuman-Witherspoon solution. Because both of the solutions present equally good visual matches and generate equally low standard errors, the representative upper limit of the confining unit vertical hydraulic conductivity near well W379 EM is estimated to be an average of the two solutions, or 0.0473 gpd/ft².

Table 12
Estimation of the Confining Unit Vertical Hydraulic Conductivity Near Well W379 EM¹

| Analysis Method | Confining Unit Vertical Hydraulic Conductivity [gpd/ft ²] | Confining Unit Vertical Hydraulic Conductivity/Storativity Product [gpd/ft ²] |
|-----------------------------|---|---|
| Hantush-Jacob (1955) | 0.0695 | Not Applicable |
| Hantush (1960) ² | Not Applicable | 0.0000153 |
| Neuman-Witherspoon (1969) | 0.0250 | Not Applicable |
| Representative Value | 0.0473 | Not Applicable |

¹ The values that are presented in this table are upper limit values only. This is because unique solutions could not be obtained for the parameters used to calculate these values.

² For the Hantush Method (1960), only the product of confining unit vertical hydraulic conductivity and confining unit storativity can be calculated.

LADWP Well W389 EM (Big Pine Wellfield)

One observation well, well T736, was monitored in addition to the pumping well during the aquifer test conducted at well W389 EM. Observation measurements from well T736 were chosen for analysis, and all of the measurements were analyzed, (except for data collected less than 10 minutes into the test). A summary of the AQTESOLV inputs used to conduct the aquifer test analysis for well W389 EM was presented in **Table 2**, and the drawdown vs. time data that was used for this analysis is located in **Appendix C**. **Table 13** presents a summary of the aquifer test analysis results for well W389 EM. The final solution curves for each analysis method are presented in **Appendix I**.

It is important to note that the top 30 feet of the screened interval for well W387 EM are located in a potential confining/low permeability unit, (greenish blue silt and clay with medium sand lenses). Because the well was screened in this area, this 30-foot low permeability unit was considered to be part of the confined aquifer. (AQTESOLV expects the screened interval of a pumping well to begin below the confining unit.) However, if analysis was performed assuming the low permeability unit to be part of the confining unit and assuming the screened interval to start immediately below this unit, different results might be obtained.

Table 13
Summary of Aquifer Test Analysis Results for Well W389 EM

| Analysis Results | Aquifer Test Analysis Method | | |
|--|------------------------------|-----------------------|---|
| | Hantush-Jacob (1955) | Hantush (1960) | Neuman-Witherspoon (1969) |
| Residual Statistics¹ | | | |
| Mean | 0.0000842 | 0.00000821 | -0.004598 |
| Variance | 0.08938 | 0.0181 | 0.09644 |
| Standard Error | 0.299 | 0.1345 | 0.3105 |
| Parameter Solutions² | | | |
| T [gpd/ft] | 90,200 | 82,900 | 90,700 |
| S [unitless] | 1.61×10^{-3} | 1.76×10^{-3} | 1.57×10^{-3} |
| r/B [unitless] | 6.05×10^{-3} | Not Applicable | 5.10×10^{-3} |
| β_H [unitless] | Not Applicable | 7.17×10^{-3} | Not Applicable |
| β_{NW} [unitless] | Not Applicable | Not Applicable | 1.40×10^{-4} |
| T ^U [gpd/ft] | Not Applicable | Not Applicable | 3.91×10^{-1} |
| S ^U [unitless] | Not Applicable | Not Applicable | 8.18×10^{-1} |

¹ Residual statistics presented for the Hantush Method should not be compared with values from the other two methods as this analysis was performed only on a subset of the data used for analysis with the other two methods.

² Bolded parameters can be varied over orders of magnitude without affecting the curve shape.

Hantush-Jacob (1955) Analysis

Analysis with the Hantush-Jacob Method produced a solution curve that provides a very good visual match with the observation data collected at well T734, including late-time data. Calculated transmissivity and storativity values agree well with earlier estimates by the LADWP (LADWP, 1993). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity.

Hantush (1960) Analysis

Analysis with the Hantush Method was conducted only on the first 210 minutes of data, (excluding the first 10 minutes). This analysis produced a solution curve that provides a very good visual match with the observation data, and calculated transmissivity and storativity values agree well with earlier LADWP estimates (LADWP, 1993). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity/storativity product.

Neuman-Witherspoon (1969) Analysis

Analysis with the Neuman-Witherspoon Method produced a solution curve that provides a very good visual match with the observation data, including late-time data. Calculated confined aquifer transmissivity and storativity values agree well with earlier LADWP estimates (LADWP, 1993). Additionally, the curve shape is sensitive to the parameters needed to calculate the

confining unit vertical hydraulic conductivity. (However, it should be noted that the calculated β_{NW} , unconfined aquifer transmissivity, and unconfined aquifer storativity values can be varied over many orders of magnitude with no visual effect on the curve shape.)

Confining Unit Vertical Hydraulic Conductivity Near Well W389 EM

Calculated vertical hydraulic conductivity values for the confining unit present at well W389 EM are presented in **Table 14**. From this table, it is observed that the confining unit vertical hydraulic conductivity values calculated from the Hantush-Jacob solution and the Neuman-Witherspoon solution are virtually identical. The representative confining unit vertical hydraulic conductivity near well W389 EM is estimated to be an average of the two, or 0.133 gpd/ft².

Table 14
Estimation of the Confining Unit Vertical Hydraulic Conductivity Near Well W389 EM

| Analysis Method | Confining Unit Vertical Hydraulic Conductivity [gpd/ft ²] | Confining Unit Vertical Hydraulic Conductivity/Storativity Product [gpd/ft ²] |
|-----------------------------|---|---|
| Hantush-Jacob (1955) | 0.155 | Not Applicable |
| Hantush (1960) ¹ | Not Applicable | 0.00563 |
| Neuman-Witherspoon (1969) | 0.110 | Not Applicable |
| Representative Value | 0.133 | Not Applicable |

¹ For the Hantush Method (1960), only the product of confining unit vertical hydraulic conductivity and confining unit storativity can be calculated.

LADWP Well W383 EM (Independence-Oak Wellfield)

One observation well, well T632, was monitored in addition to the pumping well during the aquifer test conducted at well W383 EM. Observation measurements from well T632 were chosen for analysis, and all of the measurements were analyzed, (except for data collected less than 10 minutes into the test). A summary of the AQTESOLV inputs used to conduct the aquifer test analysis for well W383 EM was presented in **Table 2**, and the drawdown vs. time data that was used for this analysis is located in **Appendix C**. **Table 15** presents a summary of the aquifer test analysis results for well W383 EM. The final solution curves for each analysis method are presented in **Appendix J**.

Hantush-Jacob (1955) Analysis

Analysis with the Hantush-Jacob Method produced a solution curve that provides a reasonable visual match with the observation data collected at well T632. However, the curve does not match the data measured at time greater than 1000 minutes, nor does it match the data measured at time less than 15 minutes. Calculated transmissivity and storativity values agree well with earlier estimates by the LADWP (LADWP, 1992). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity.

Table 15
Summary of Aquifer Test Analysis Results for Well W383 EM

| Analysis Results | Aquifer Test Analysis Method | | |
|--|------------------------------|-----------------------|---|
| | Hantush-Jacob (1955) | Hantush (1960) | Neuman-Witherspoon (1969) |
| Residual Statistics¹ | | | |
| Mean | 0.0000617 | -0.0003572 | -0.0004633 |
| Variance | 0.06749 | 0.03992 | 0.01913 |
| Standard Error | 0.2598 | 0.1998 | 0.1383 |
| Parameter Solutions² | | | |
| T [gpd/ft] | 56,400 | 43,100 | 50,000 |
| S [unitless] | 4.88×10^{-4} | 6.05×10^{-4} | 5.89×10^{-4} |
| r/B [unitless] | 4.64×10^{-2} | Not Applicable | 6.96×10^{-2} |
| β_H [unitless] | Not Applicable | 4.27×10^{-2} | Not Applicable |
| β_{NW} [unitless] | Not Applicable | Not Applicable | 4.44×10^{-3} |
| T ^U [gpd/ft] | Not Applicable | Not Applicable | 1.08×10^{-1} |
| S ^U [unitless] | Not Applicable | Not Applicable | 1.06×10^{-2} |

¹ Residual statistics presented for the Hantush Method should not be compared with values from the other two methods as this analysis was performed only on a subset of the data used for analysis with the other two methods.

² Bolded parameters can be varied over orders of magnitude without affecting the curve shape.

Hantush (1960) Analysis

Analysis with the Hantush Method was conducted only on the first 100 minutes of data, (excluding the first 10 minutes). This analysis produced a solution curve that provides a relatively good visual match with the observation data, and calculated transmissivity and storativity values agree well with earlier LADWP estimates (LADWP, 1992). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity/storativity product.

Neuman-Witherspoon (1969) Analysis

Analysis with the Neuman-Witherspoon Method produced a solution curve that provides the best visual match of the three analysis methods with the observation data, matching both early- and late-time data. Calculated confined aquifer transmissivity and storativity values agree well with earlier LADWP estimates (LADWP, 1992). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity. (However, it should be noted that the calculated β_{NW} and the unconfined aquifer transmissivity values can be varied over many orders of magnitude with no visual effect on the curve shape.)

Confining Unit Vertical Hydraulic Conductivity Near Well W383 EM

Calculated vertical hydraulic conductivity values for the confining unit present at well W383 EM are presented in **Table 16**. From this table, it is observed that the confining unit vertical hydraulic

conductivity value calculated from the Neuman-Witherspoon solution is approximately two times greater than the value calculated from the Hantush-Jacob solution. Because the Neuman-Witherspoon solution presents a higher quality visual match as well as a lower standard error, the representative confining unit vertical hydraulic conductivity near well W383 EM is expected to be closer to the value calculated with the Neuman-Witherspoon solution, and is estimated to be 5.47 gpd/ft².

Table 16
Estimation of the Confining Unit Vertical Hydraulic Conductivity Near Well W383 EM

| Analysis Method | Confining Unit Vertical Hydraulic Conductivity [gpd/ft ²] | Confining Unit Vertical Hydraulic Conductivity/Storativity Product [gpd/ft ²] |
|-----------------------------|---|---|
| Hantush-Jacob (1955) | 2.74 | Not Applicable |
| Hantush (1960) ¹ | Not Applicable | 0.0172 |
| Neuman-Witherspoon (1969) | 5.47 | Not Applicable |
| Representative Value | 5.47 | Not Applicable |

¹ For the Hantush Method (1960), only the product of confining unit vertical hydraulic conductivity and confining unit storativity can be calculated.

LADWP Well W384 EM (Independence-Oak Wellfield)

One observation well, well T633, was monitored in addition to the pumping well during the aquifer test conducted at well W384 EM. Observation measurements from well T633 were chosen for analysis, and all of the measurements were analyzed, (except for data collected less than 10 minutes into the test). A summary of the AQTESOLV inputs used to conduct the aquifer test analysis for well W384 EM was presented in **Table 2**, and the drawdown vs. time data that was used for this analysis is located in **Appendix C**. **Table 17** presents a summary of the aquifer test analysis results for well W384 EM. The final solution curves for each analysis method are presented in **Appendix K**.

It is important to note that the top 40 feet of the screened interval for well W387 EM are located in a potential confining/low permeability unit, (sand and gravel, cemented). Because the well was screened in this area, this 40-foot low permeability unit was considered to be part of the confined aquifer. (AQTESOLV expects the screened interval of a pumping well to begin below the confining unit.) However, if analysis was performed assuming the low permeability unit to be part of the confining unit and assuming the screened interval to start immediately below this unit, different results might be obtained.

Hantush-Jacob (1955) Analysis

Analysis with the Hantush-Jacob Method produced a solution curve that provides an acceptable visual match with the observation data collected at well T633. However, the curve does not match the late-time data well, nor does it match data measured at time less than 15 minutes.

Calculated transmissivity and storativity values agree well with earlier estimates by the LADWP (LADWP, 1993). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity.

Table 17
Summary of Aquifer Test Analysis Results for Well W384 EM

| Analysis Results | Aquifer Test Analysis Method | | |
|--|------------------------------|-----------------------|---|
| | Hantush-Jacob (1955) | Hantush (1960) | Neuman-Witherspoon (1969) |
| Residual Statistics¹ | | | |
| Mean | -0.0000309 | -0.000170 | -0.0004342 |
| Variance | 0.04908 | 0.007146 | 0.007922 |
| Standard Error | 0.2215 | 0.08453 | 0.08901 |
| Parameter Solutions² | | | |
| T [gpd/ft] | 36,600 | 27,200 | 24,300 |
| S [unitless] | 1.97×10^{-4} | 2.79×10^{-4} | 3.02×10^{-4} |
| r/B [unitless] | 2.77×10^{-2} | Not Applicable | 1.17×10^{-1} |
| β_H [unitless] | Not Applicable | 3.58×10^{-2} | Not Applicable |
| β_{NW} [unitless] | Not Applicable | Not Applicable | 5.68×10^{-2} |
| T^U [gpd/ft] | Not Applicable | Not Applicable | 1.08×10^{-1} |
| S^U [unitless] | Not Applicable | Not Applicable | 7.42×10^{-3} |

¹ Residual statistics presented for the Hantush Method should not be compared with values from the other two methods as this analysis was performed only on a subset of the data used for analysis with the other two methods.

² Bolded parameters can be varied over orders of magnitude without affecting the curve shape.

Hantush (1960) Analysis

Analysis with the Hantush Method was conducted only on the first 200 minutes of data, (excluding the first 10 minutes). This analysis produced a solution curve that provides a good visual match with the observation data, and calculated transmissivity and storativity values agree well with earlier LADWP estimates (LADWP, 1993). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity/storativity product.

Neuman-Witherspoon (1969) Analysis

Analysis with the Neuman-Witherspoon Method produced a solution curve that provides the best visual match of the three analysis methods with the observation data. Calculated confined aquifer transmissivity and storativity values agree well with earlier LADWP estimates (LADWP, 1993). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity. (However, it should be noted that the calculated unconfined aquifer transmissivity can be varied over many orders of magnitude with no visual effect on the curve shape.)

Confining Unit Vertical Hydraulic Conductivity Near Well W384 EM

Calculated vertical hydraulic conductivity values for the confining unit present at well W384 EM are presented in **Table 18**. From this table, it is observed that the confining unit vertical hydraulic conductivity value calculated from the Neuman-Witherspoon solution is approximately one order of magnitude greater than the value calculated from the Hantush-Jacob solution. Because the Neuman-Witherspoon solution presents a higher quality visual match as well as a lower standard error, the representative confining unit vertical hydraulic conductivity near well W384 EM is expected to be closer to the value calculated with the Neuman-Witherspoon solution, and is estimated to be 2.11 gpd/ft².

Table 18
Estimation of the Confining Unit Vertical Hydraulic Conductivity Near Well W384 EM

| Analysis Method | Confining Unit Vertical Hydraulic Conductivity [gpd/ft ²] | Confining Unit Vertical Hydraulic Conductivity/Storativity Product [gpd/ft ²] |
|-----------------------------|---|---|
| Hantush-Jacob (1955) | 0.180 | Not Applicable |
| Hantush (1960) ¹ | Not Applicable | 0.000992 |
| Neuman-Witherspoon (1969) | 2.11 | Not Applicable |
| Representative Value | 2.11 | Not Applicable |

¹ For the Hantush Method (1960), only the product of confining unit vertical hydraulic conductivity and confining unit storativity can be calculated.

LADWP Well W395 AQ (Symmes-Shepherd Wellfield)

Two observation wells, well T782 and well W394 AQ, were monitored in addition to the pumping well during the aquifer test conducted at well W395 AQ. However, upon looking at the drawdown vs. time plots for both observation wells, it was determined that both of these wells seem to be affected by some outside pumping source and/or external influences, due to significant variation in observed drawdown at late time. For this reason, only the pumping well data from well W395 AQ were chosen for analysis, and all of the measurements were analyzed, (except for data collected less than 10 minutes into the test). A summary of the AQTESOLV inputs used to conduct the aquifer test analysis for well W395 AQ was presented in **Table 2**, and the drawdown vs. time data that was used for this analysis is located in **Appendix C**. **Table 19** presents a summary of the aquifer test analysis results for well W395 AQ. The final solution curves for each analysis method are presented in **Appendix L**.

Hantush-Jacob (1955) Analysis

Analysis with the Hantush-Jacob Method produced a solution curve that provides a very good visual match with all of the pumping well data collected at well W395 AQ. Calculated transmissivity and storativity values agree with earlier estimates by the LADWP (LADWP, 1992).

Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity.

Table 19
Summary of Aquifer Test Analysis Results for Well W395 AQ

| Analysis Results | Aquifer Test Analysis Method | | |
|--|------------------------------|-----------------------|------------------------------|
| | Hantush-Jacob (1955) | Hantush (1960) | Neuman-Witherspoon (1969) |
| Residual Statistics¹ | | | |
| Mean | 0.0000163 | -0.0004988 | -0.00000601 |
| Variance | 0.02339 | 0.01451 | 0.02166 |
| Standard Error | 0.153 | 0.1205 | 0.1472 |
| Parameter Solutions² | | | |
| T [gpd/ft] | 245,800 | 235,700 | 225,200 |
| S [unitless] | 7.49×10^{-4} | 1.23×10^{-3} | 1.27×10^{-3} |
| r/B [unitless] | 1.02×10^{-4} | Not Applicable | 2.30×10^{-4} |
| β_H [unitless] | Not Applicable | 2.69×10^{-5} | Not Applicable |
| β_{NW} [unitless] | Not Applicable | Not Applicable | 4.24×10^{-5} |
| T ^U [gpd/ft] | Not Applicable | Not Applicable | 1.69×10^{-5} |
| S ^U [unitless] | Not Applicable | Not Applicable | 1.32×10^{-3} |

¹ Residual statistics presented for the Hantush Method should not be compared with values from the other two methods as this analysis was performed only on a subset of the data used for analysis with the other two methods.

² Bolded parameters can be varied over orders of magnitude without affecting the curve shape.

Hantush (1960) Analysis

Analysis with the Hantush Method was conducted only on the first 1,260 minutes of data, (excluding the first 10 minutes). This analysis produced a solution curve that provides a very good visual match with the pumping well data, and calculated transmissivity and storativity values agree with earlier LADWP estimates (LADWP, 1992). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity/storativity product.

Neuman-Witherspoon (1969) Analysis

Analysis with the Neuman-Witherspoon Method produced a solution curve that provides a very good visual match with all of the pumping well data. Calculated confined aquifer transmissivity and storativity values agree with earlier LADWP estimates (LADWP, 1992). Additionally, the curve shape is sensitive to the parameters needed to calculate the confining unit vertical hydraulic conductivity.

Confining Unit Vertical Hydraulic Conductivity Near Well W395 AQ

Calculated vertical hydraulic conductivity values for the confining unit present at well W395 AQ are presented in **Table 20**. From this table, it is observed that the confining unit vertical hydraulic conductivity value calculated from the Hantush-Jacob solution is approximately five times greater than the value calculated from the Neuman-Witherspoon solution. Because both of the solutions present equally good visual matches and generate equally low standard errors, the representative confining unit vertical hydraulic conductivity near well W395 AQ is estimated to be an average of the two solutions, or 0.158 gpd/ft².

Table 20
Estimation of the Confining Unit Vertical Hydraulic Conductivity Near Well W395 AQ

| Analysis Method | Confining Unit Vertical Hydraulic Conductivity [gpd/ft ²] | Confining Unit Vertical Hydraulic Conductivity/Storativity Product [gpd/ft ²] |
|-----------------------------|---|---|
| Hantush-Jacob (1955) | 0.0560 | Not Applicable |
| Hantush (1960) ¹ | Not Applicable | 0.0000736 |
| Neuman-Witherspoon (1969) | 0.260 | Not Applicable |
| Representative Value | 0.158 | Not Applicable |

¹ For the Hantush Method (1960), only the product of confining unit vertical hydraulic conductivity and confining unit storativity can be calculated.

SUMMARY OF FINDINGS

Table 21 presents a comparison between the confined aquifer transmissivity and storativity values calculated during this analysis and the existing LADWP estimates. Also included in this table are the estimated representative vertical hydraulic conductivity values of the confining unit based on the analyses presented in this report.

Calculated aquifer transmissivity ranges from 20,000 gpd/ft near well V016GA (Big Pine Wellfield) up to 246,000 gpd/ft at Well W395 (Symmes-Shepherd Wellfield), spanning an order of magnitude. Previous LADWP estimates for transmissivity were presented as either ranges or as a single value. For the six pump test packets where ranges were presented, 16 of the 18 new transmissivity estimates fall within the previously estimated ranges. The two values that lie outside of the previously estimated ranges are both associated with well W384 EM in the Independence-Oak Wellfield. Both estimates are within a factor of two of the range's lower bound. For the three pump test packets that present single transmissivity estimates, all new transmissivity values are within a factor of three of the existing estimates.

Table 21
Summary of Aquifer and Confining Unit Parameters¹

| | | Laws | | Big Pine | | | | Independence-Oak | | Symmes-Shepherd |
|--|--|---|---|-------------------------------|-------------------------------------|---|--|----------------------------|-------------------------------|---|
| | | W387 EM | W398 AQ | V014GA | V016GA | W379 EM | W389 EM | W383 EM | W384 EM | W395 AQ |
| Confined Aquifer Parameters ² | | | | | | | | | | |
| T (gpd/ft) | Existing LADWP Estimate | 50,000-80,000 | 120,000 | 36,000 | 45,000 | 80,000-90,000 | 80,000-100,000 | 40,000-60,000 | 35,000-40,000 | 220,000-300,000 |
| | AQTESOLV Value – Hantush-Jacob (1955) | 57,000 | 82,000 | 41,000 | 26,000 | 90,000 | 90,000 | 56,000 | 37,000 | 246,000 |
| | AQTESOLV Value – Hantush (1960) | 57,000 | 74,000 | 36,000 | 24,000 | 90,000 | 83,000 | 43,000 | 27,000 | 236,000 |
| | AQTESOLV Value – Neuman-Witherspoon (1969) | 54,000 | 71,000 | 33,000 | 20,000 | 90,000 | 91,000 | 50,000 | 24,000 | 225,000 |
| S (unitless) | Existing LADWP Estimate | 3 x 10 ⁻³ | 2 x 10 ⁻³ | Not Applicable | Not Applicable | 1 x 10 ⁻³ | 1 x 10 ⁻³ | 7 x 10 ⁻⁴ | 2 x 10 ⁻⁴ | 3 x 10 ⁻⁴ |
| | AQTESOLV Value – Hantush-Jacob (1955) | 4 x 10 ⁻³ | 6 x 10⁻⁴ | 3 x 10 ⁻³ | 3 x 10⁻⁴ | 1 x 10 ⁻³ | 2 x 10 ⁻³ | 5 x 10 ⁻⁴ | 2 x 10 ⁻⁴ | 7 x 10 ⁻⁴ |
| | AQTESOLV Value – Hantush (1960) | 4 x 10 ⁻³ | 1 x 10⁻³ | 3 x 10 ⁻² | 6 x 10⁻⁴ | 1 x 10 ⁻³ | 2 x 10 ⁻³ | 6 x 10 ⁻⁴ | 3 x 10 ⁻⁴ | 1 x 10 ⁻³ |
| | AQTESOLV Value – Neuman-Witherspoon (1969) | 3 x 10 ⁻³ | 1 x 10⁻³ | 1 x 10 ⁻³ | 2 x 10⁻³ | 1 x 10 ⁻³ | 2 x 10 ⁻³ | 6 x 10 ⁻⁴ | 3 x 10 ⁻⁴ | 1 x 10 ⁻³ |
| Analysis Method for Existing LADWP Estimates | | Modified Theis, Hantush, Modified Hantush | Modified Theis, Hantush | Modified Theis | Modified Theis | Modified Theis, Hantush, Modified Hantush | Modified Theis | Modified Theis, Hantush | Modified Theis | Modified Theis, Hantush, Modified Hantush |
| Confining Unit Parameters | | | | | | | | | | |
| Confining Unit Materials Based on Well Log Descriptions | | Fine med sand, gravel, and clay | Fine to medium sand with clay and cobbles | Sand, silt, and tight clay | Silty sand, silt, and tight clay | Clay | Silt , clay, and gravel/sand lenses | Sand, clay, gravel | Sand, cemented, black rock | Red sticky clay mixed with fine sand |
| Representative Aquitard Vertical Hydraulic Conductivity | gpd/ft ² | 3.64 | 2.41 | 3.52 | 24.4 | 0.0473 | 0.133 | 5.47 | 2.11 | 0.158 |
| | ft/day | 0.486 | 0.322 | 0.470 | 3.26 | 0.00632 | 0.0177 | 0.731 | 0.282 | 0.0211 |

¹ Bolded numbers represent estimated values to be used with caution because AQTESOLV could not converge on solutions for these data sets.
² In this table, transmissivity and storativity values estimated during this analysis have been rounded to the same number of significant figures as the existing LADWP estimates for the purpose of comparison.

The calculated storativity values vary from 3×10^{-2} at well V014GA (Big Pine Wellfield) to 2×10^{-4} at well W384 EM (Independence-Oak Wellfield), spanning approximately two orders of magnitude. All calculated storativity values demonstrate order of magnitude agreement with previous LADWP estimates. In fact, all storativity values estimated during this analysis are within a factor of four of the existing LADWP estimates.

The calculated confining unit vertical hydraulic conductivity values vary from as low as 0.0473 gpd/ft² at well W379 EM (Big Pine Wellfield) to as high as 24.4 gpd/ft² at well V016GA (Big Pine Wellfield). An average value was calculated to be approximately 4.65 gpd/ft². According to Hollett and others (1991), vertical hydraulic conductivity for the confining unit described as Hydrostratigraphic Unit 2 ranges from 0.006 gpd/ft² to 0.015 gpd/ft².

The aquifer parameters shown in **Table 21** provide guidance on the typical aquifer parameters found in the Owens Valley, as well as estimated values at specific locations. It is clear from the lithologic data presented in **Appendix B** that the nature of the confining unit is highly variable across the Valley. Only in rare cases (such as the lower portion of the confining unit at V014GA) does the confining unit consist purely of typical low-permeability materials such as “tight clay”. In most cases, the confining unit consists of stratified clays, silts, sands, and even gravels. Because horizontal hydraulic conductivities may exceed vertical hydraulic conductivities by a factor of 100 in highly stratified material (Walton, 1988), “leaky”-type behavior is expected even in the absence of an obvious confining unit.

Comparison of the lithologic description to the calculated vertical hydraulic conductivity of the confining unit does not reveal a consistent, reliable pattern of lithologic description and computed vertical hydraulic conductivity. The absence of a consistent, reliable pattern is believed to be the result of lithologic descriptions (as reported on well construction logs) that are not detailed or consistent enough for valid comparison, thereby highlighting the need for careful and detailed lithologic logging by a qualified geologist during drilling.

It is important to note that because the lithologic logs contain the only information available in order to estimate thicknesses for the confining unit and for the confined aquifer for each location being examined, the level of confidence in the results obtained during this analysis is directly related to the level of confidence placed on the lithologic logs themselves. Confined aquifer thickness and especially confining unit thickness are important parameters needed to ultimately determine the vertical hydraulic conductivity of the confining unit. Re-definition of these parameters based on more accurate lithologic information and additional geophysical information could have a significant effect on the hydraulic conductivity values obtained during this study.

RECOMMENDATIONS FOR FUTURE AQUIFER TESTING

The analysis of previous aquifer tests has resulted not only in improved estimates of the transmissivity and storage coefficient of the confined aquifer, but also what are believed to be

reasonable estimates of the vertical hydraulic conductivity of the overlying confining unit. Future aquifer testing in the Owens Valley could be improved by the recommendations presented herein:

- In many cases, the lithologic descriptions of the penetrated materials are generalized, and lack sufficient detail. Lithologic generalizations make it difficult to determine the exact nature of the confining unit and aquifer. During the construction of future wells, careful lithologic logging conducted by a qualified geologist, preferably under the supervision of a Registered Geologist in the State of California, is recommended. In addition, geophysical logging is also recommended.
- During constant-rate aquifer tests, the pumping rate should be kept constant at all times. Evaluation of the nine tests demonstrated that even small changes in pumping rate produce jumps in observed drawdown that are difficult to reconcile during analysis of the results. (See **Appendix H**, which presents the results for well W379 EM for an example.) Control of the pumping rate is facilitated by the use of a flow valve and instantaneous-read flow meter with totalizer.
- Automated dataloggers should be utilized whenever possible (at least in the pumping well) to detect rapidly increasing drawdown during the initial portion of the test. Data loggers are also valuable in detecting variations in pumping rate in the pumped well.
- The length of the constant-rate test must be sufficiently long to observe the late-time effects of leakage if reliable estimates of confining unit properties are to be generated. In general, data obtained after 1,000 minutes (after about 17 hours) was crucial in identifying the leakage factor with both the Hantush-Jacob Method and the Neuman-Witherspoon Method. Late-time data are not needed to create a good match with the Hantush Method; however, this method is significantly less valuable for determining confining layer properties, as this method yields only the product of confining unit vertical hydraulic conductivity and confining unit storativity. For this reason, aquifer test durations of at least 24 hours are recommended, and much longer-term duration tests may be advantageous, depending on the drawdown pattern observed during testing, and the distance to observation wells.
- Because all transient analysis methods used in this report are very sensitive to the radius of observation when the radius is low, and the methods assume a well of infinitesimal diameter, observation wells should be monitored in addition to monitoring drawdown in the pumping well. Ideally, the observation wells should be screened in the same aquifer as the pumping well, and multiple observation wells at various azimuths and distances would be ideal in order to evaluate the horizontal anisotropy and heterogeneity of the aquifer. In general, a more representative and regional evaluation can be made with multiple observation wells at varying distances.
- If feasible and practical, aquifer tests should also include monitoring of the shallow aquifer above the confining unit. All of the methods used in this report assume that there is negligible drawdown in this aquifer, and this appears to have been documented by previous testing (Harrington, 2001). Nevertheless, the shallow aquifer should be monitored during testing to confirm this assumption. In addition, the shallow aquifer is of significant concern from an ecological perspective.

- During aquifer testing, external influences, such as turning off or on of adjacent wells and major changes in surface flow, should be avoided.

REFERENCES

AQTESOLV for Windows, Version 3.01, by HydroSOLVE, Inc.

Hantush, M. S., 1960, Modification of the Theory of Leaky Aquifers, Journal of Geophysical Research, Vol. 65, No. 11, 3713-3725.

Hantush, M. S. and Jacob, C. E., 1955, Non-Steady Radial Flow in an Infinite Leaky Aquifer, Transactions, American Geophysical Union, Vol. 36, No. 1, 95-100.

Harrington, R., 2001, Operational tests of wells 375W, 380W, 381W, and 382W: results from previous tests and recommendations for future tests and management, 37 pages.

Hollett, K. J., W. R. Danskin, W. F. McCaffrey, and C. L. Walti, 1991, Geology and Water Resources of Owens Valley, California, USGS Water-Supply Paper 2370-B, 77 pages.

LADWP Pump Test Packets:

- “E/M Well W384 EM Aquifer Test – Independence-Oak Well Field” (February, 1993)
- “Report on the Deep Well Aquifer Test at the Big Pine East of the Owens River Site” (November, 1985)
- “USGS/LA/Inyo-Owens Valley Groundwater Investigation Deep Well Aquifer Test – Big Pine West of Owens River Site” (February, 1985)
- “Well W379 EM Aquifer Test – Big Pine Well Field” (October, 1992)
- “Well W383 EM Aquifer Test – Independence-Oak Well Field” (October, 1992)
- “Well W387 EM Test – Laws Field” (May, 1993)
- “Well W389 EM Test – Big Pine Field” (September, 1993)
- “Well W395 AQ Aquifer Test – Symmes/Shepherd Well Field” (September, 1992)
- “Well W398 AQ Aquifer Test – Laws Well Field” (October, 1991)

MWH, 2002, Technical Memorandum on the Identification of Methods and Tools for Characterization of the Confining Layer, 22 pages.

Neuman, S. P. and Witherspoon, P. A., 1969, Theory of Flow in a Confined Two Aquifer System, Water Resources Research, Vol. 5, No. 4, 803-816.

Walton, W.C., 1988, Practical Aspects of Ground Water Modeling – Flow, Mass, and Heat Transport, and Subsidence – Analytical and Computer Models. National Water Well Association, Worthington, Ohio.

Appendix A

Glossary of Variables and Terms

Appendix A

Glossary of Variables and Terms

VARIABLES

| | |
|--------------|--|
| b' | Confining unit thickness |
| B | Leakage factor |
| β_H | Mathematical value calculated by the Hantush Method (This value is used to calculate the confining unit vertical hydraulic conductivity/storativity product with the Hantush Method.) |
| β_{NW} | Mathematical value calculated by the Neuman-Witherspoon Method (This value is <u>not</u> needed to calculate the confining unit vertical hydraulic conductivity with the Neuman-Witherspoon Method.) |
| K_r | Aquifer hydraulic conductivity for horizontal flow |
| K_z | Aquifer hydraulic conductivity for vertical flow |
| K_z' | Confining unit hydraulic conductivity for vertical flow |
| r | Distance from the pumped well to the observation well |
| S | Aquifer storativity |
| S' | Confined aquifer storativity (In Appendices D – L, however, S' denotes unconfined aquifer storativity for the Neuman-Witherspoon Method) |
| S^U | Unconfined aquifer storativity (In Appendices D – L, the unconfined aquifer storativity calculated by the Neuman-Witherspoon Method is denoted as S') |
| T | Aquifer transmissivity |
| T^U | Unconfined aquifer transmissivity (In Appendices D – L, the unconfined aquifer transmissivity calculated by the Neuman-Witherspoon Method is denoted as T') |

DEFINITIONS

Hydraulic Conductivity (K) is the capacity of a porous medium to transmit water. It is measured as the volume of water moving through a unit area of aquifer perpendicular to the direction of flow in unit time under a unit hydraulic gradient.

Leakage Factor (B) is a measure for the spatial distribution of the leakage through a confining unit into a leaky aquifer and vice versa. It is defined by the following equation:

$$B = \sqrt{\frac{Tb'}{K_z'}}$$

Mean as defined as the sum of the differences between each actual y-value and its predicted y-value (from the best-fit line) divided by the total number of measurements.

Appendix A: Glossary of Variables and Terms

$$\text{Mean} = \frac{\sum (y_{\text{measured}} - y_{\text{estimated}})}{N}$$

where: y_{measured} = The measured y-value

$y_{\text{estimated}}$ = The predicted y-value (from the regression line)

N = The total number of measurements

Standard Error is defined as the square root of the variance. Standard error is represented by the following equation:

$$\text{Standard Error} = \sqrt{\frac{\sum (y_{\text{measured}} - y_{\text{estimated}})^2}{N}}$$

where: y_{measured} = The measured y-value

$y_{\text{estimated}}$ = The predicted y-value (from the regression line)

N = The total number of measurements

Storativity (S) is the volume of water that a permeable unit releases or takes into storage per unit surface area of the aquifer per unit change in head.

Transmissivity (T) is the capacity of an aquifer to transmit water of the prevailing kinematic viscosity. It is measured as the rate of flow under a unit hydraulic gradient through a cross-section of aquifer having a unit width and full saturated thickness.

Variance is a measure of the dispersion or scatter of a set of values from their predicted values. Variance can be represented by the following equation:

$$\text{Variance} = \frac{\sum (y_{\text{measured}} - y_{\text{estimated}})^2}{N}$$

where: y_{measured} = The measured y-value

$y_{\text{estimated}}$ = The predicted y-value (from the regression line)

N = The total number of measurements

Appendix B
Schematic Representations of the Nine Pumping Tests
Selected for Aquifer Test Analysis

Figure B-1
Schematic Representation of Well W387 EM (Laws)

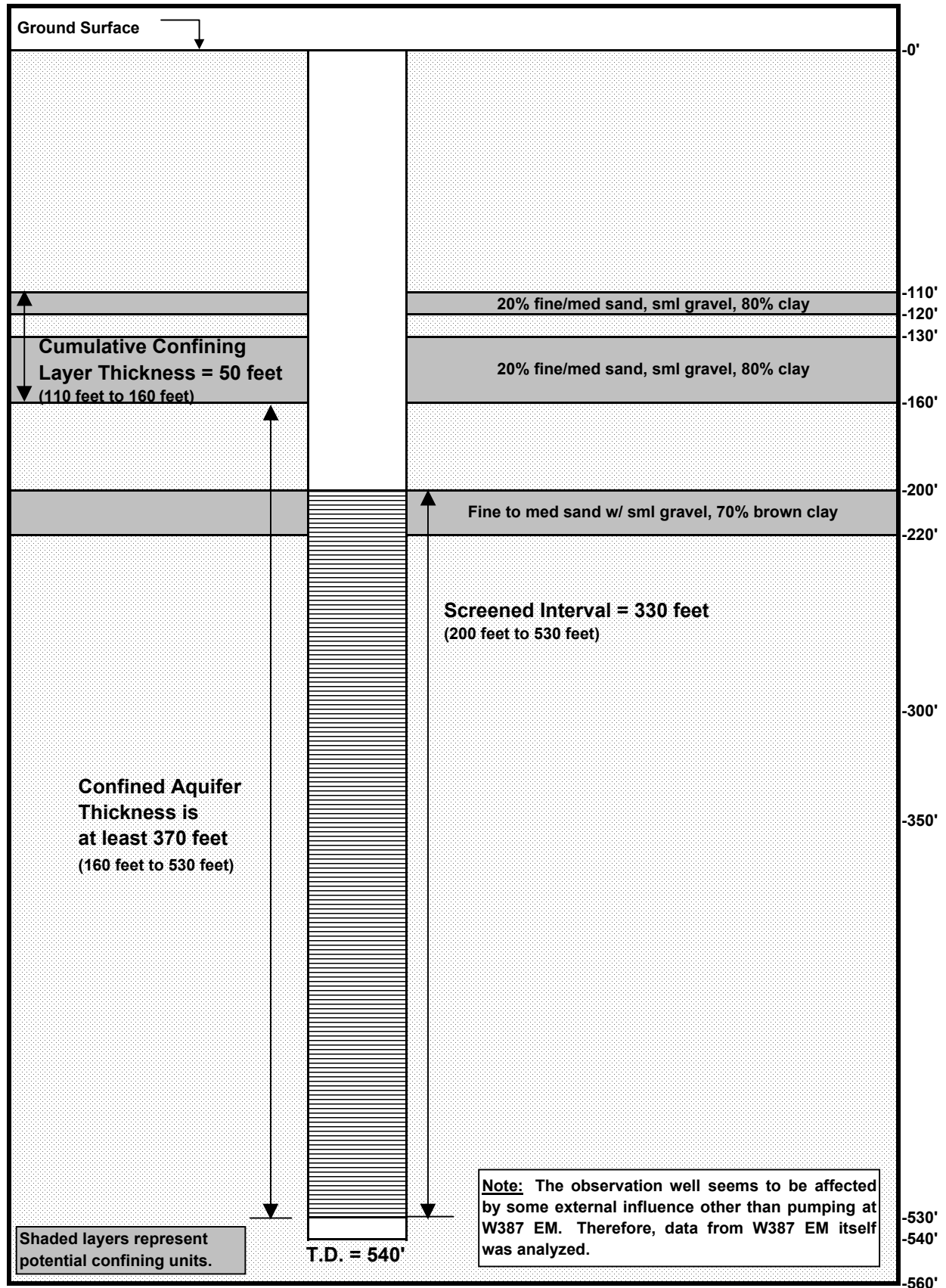


Figure B-2
Schematic Representation of Well W398 AQ (Laws)

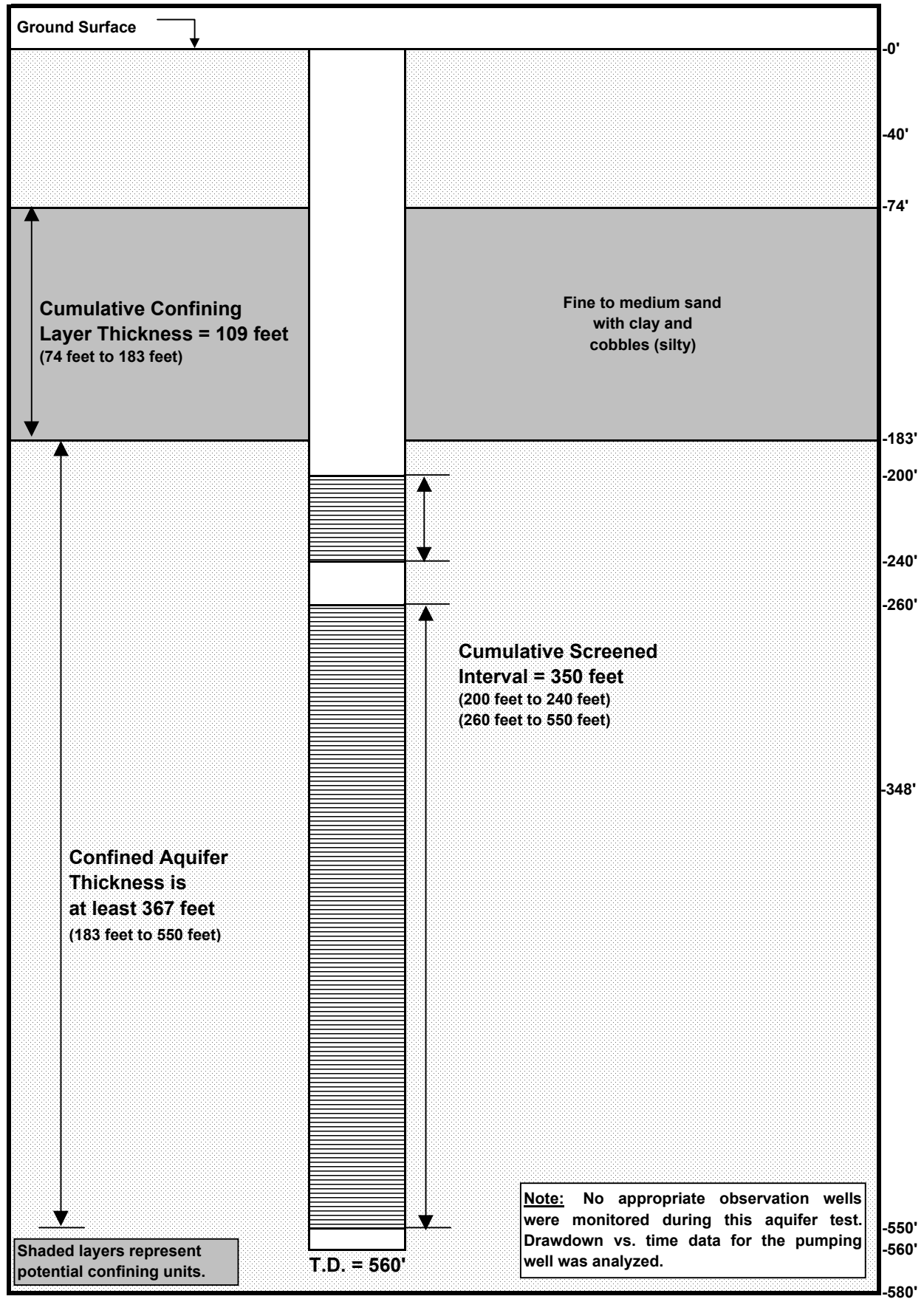


Figure B-3
Schematic Representation of Well V014GA (Big Pine)

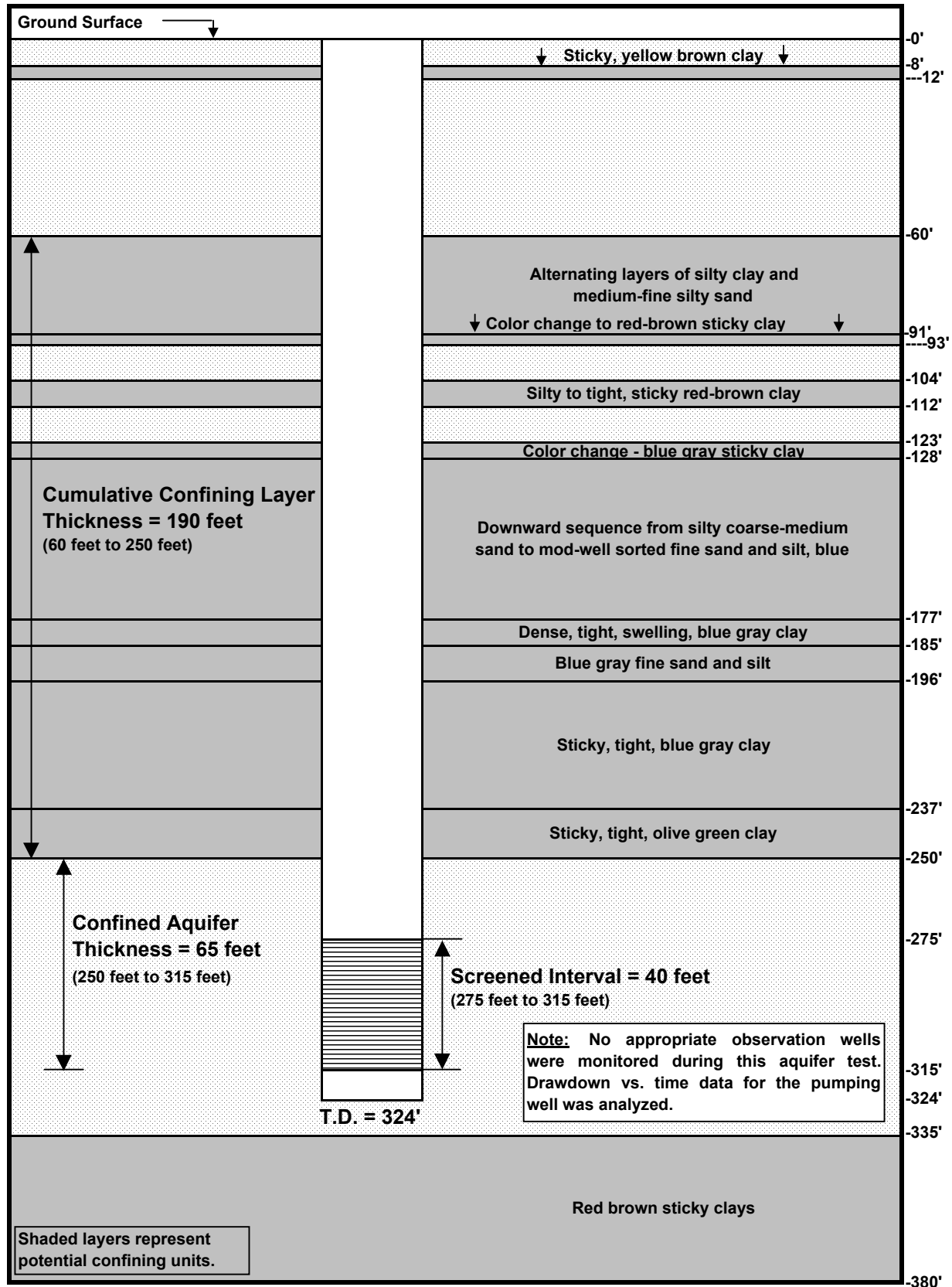


Figure B-4
Schematic Representation of Well V016GA (Big Pine)

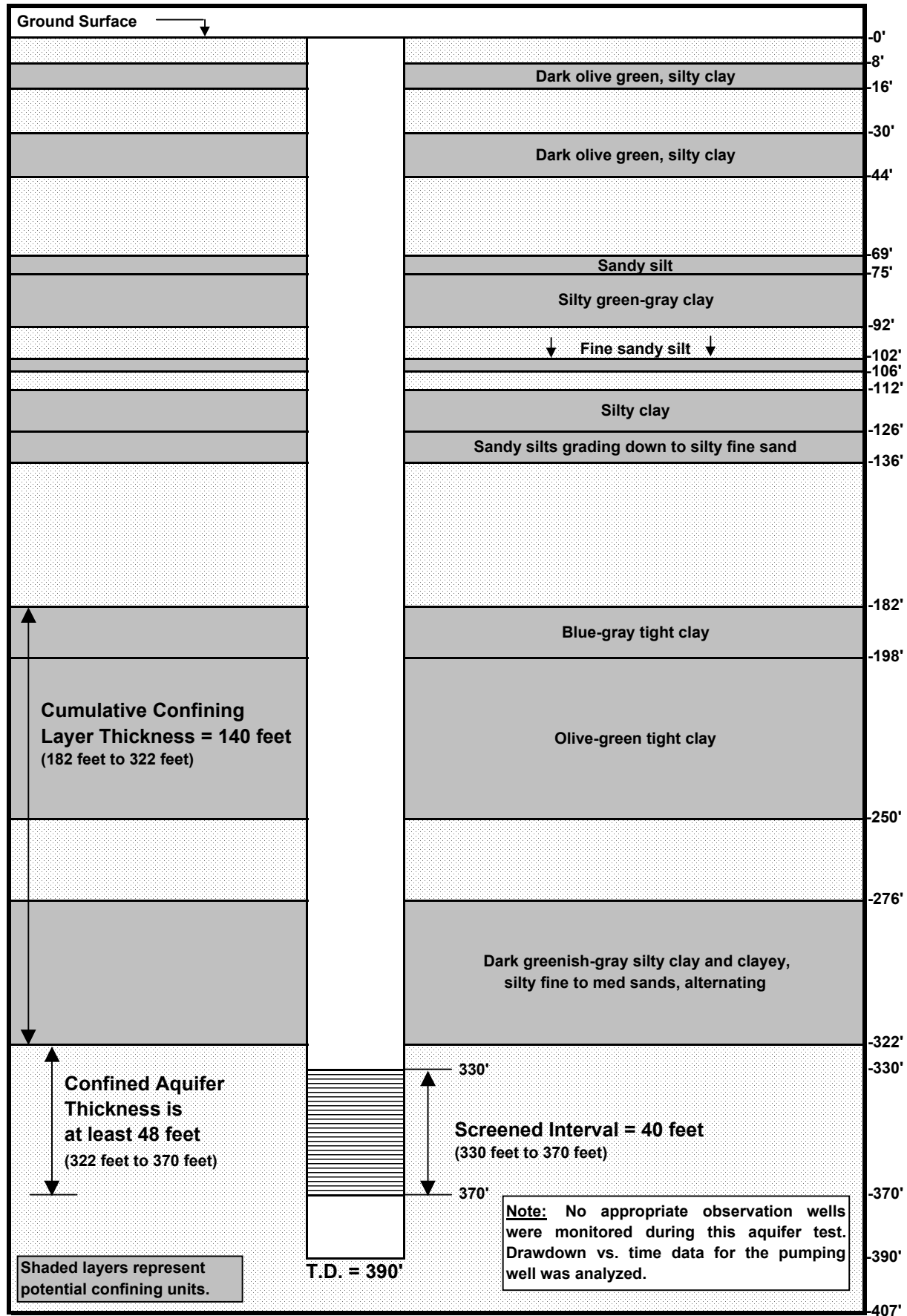


Figure B-5
Schematic Representation of Well W379 EM (Big Pine)

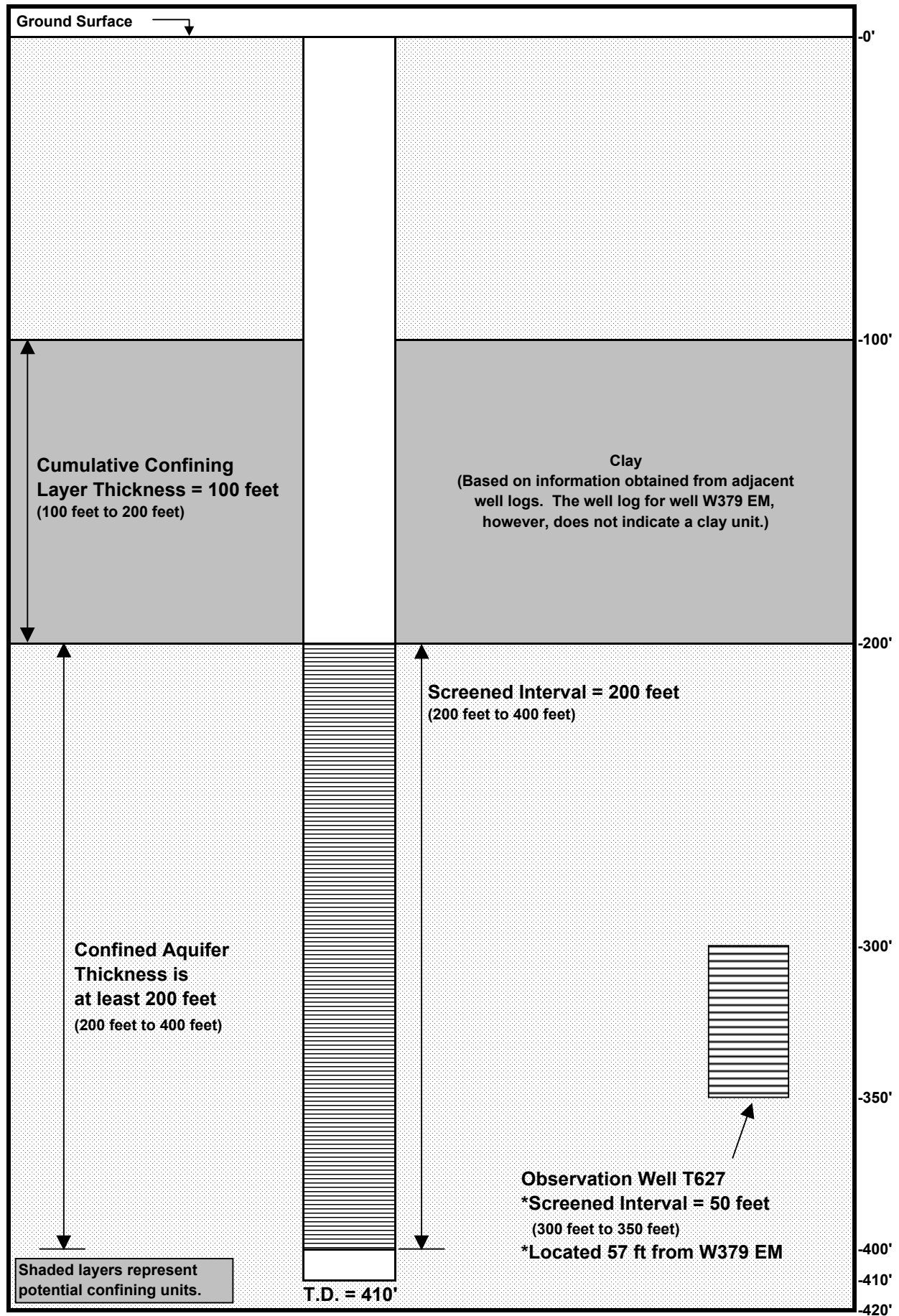


Figure B-6
Schematic Representation of Well W389 EM (Big Pine)

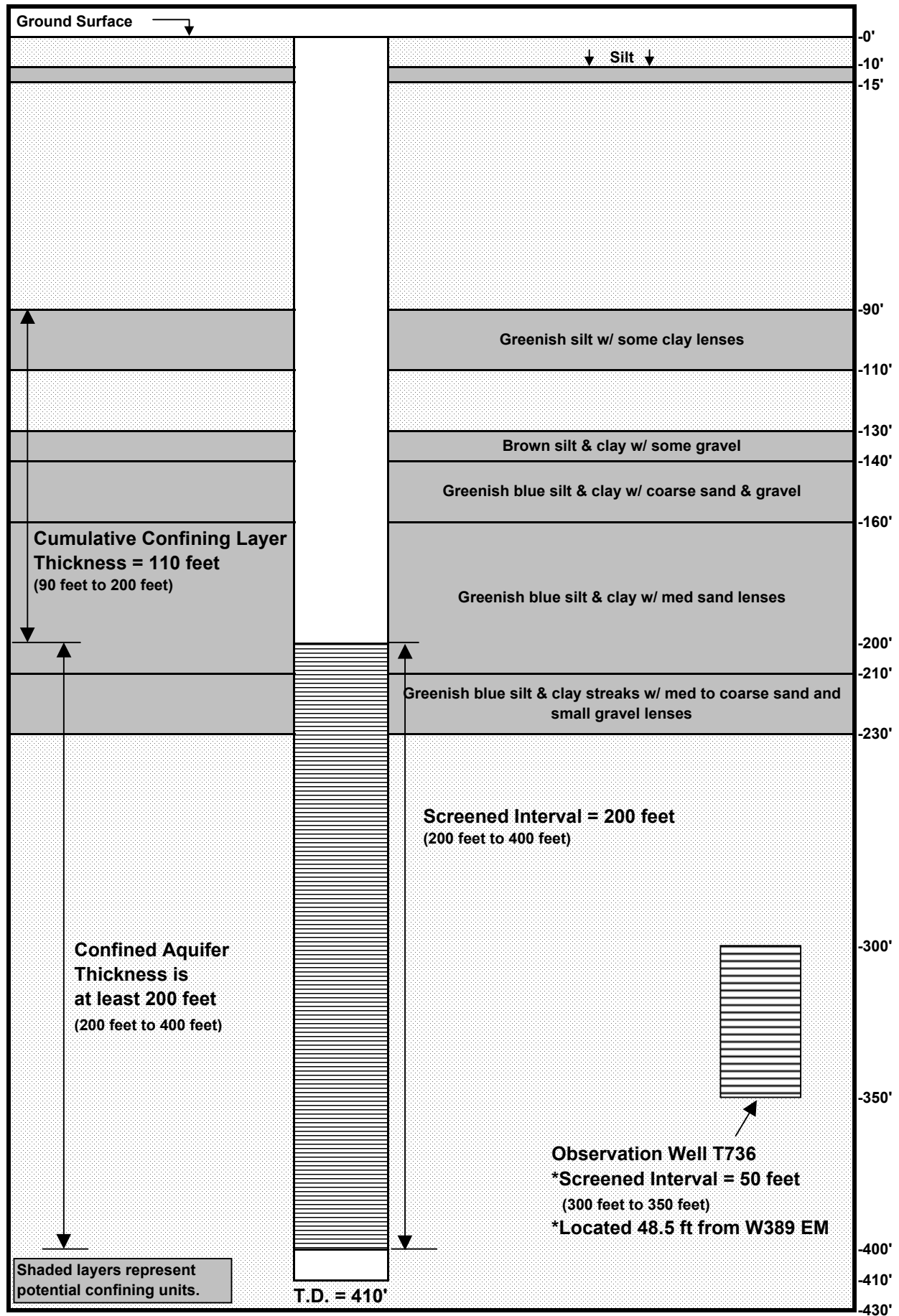


Figure B-7
Schematic Representation of Well W383 EM (Independence-Oak)

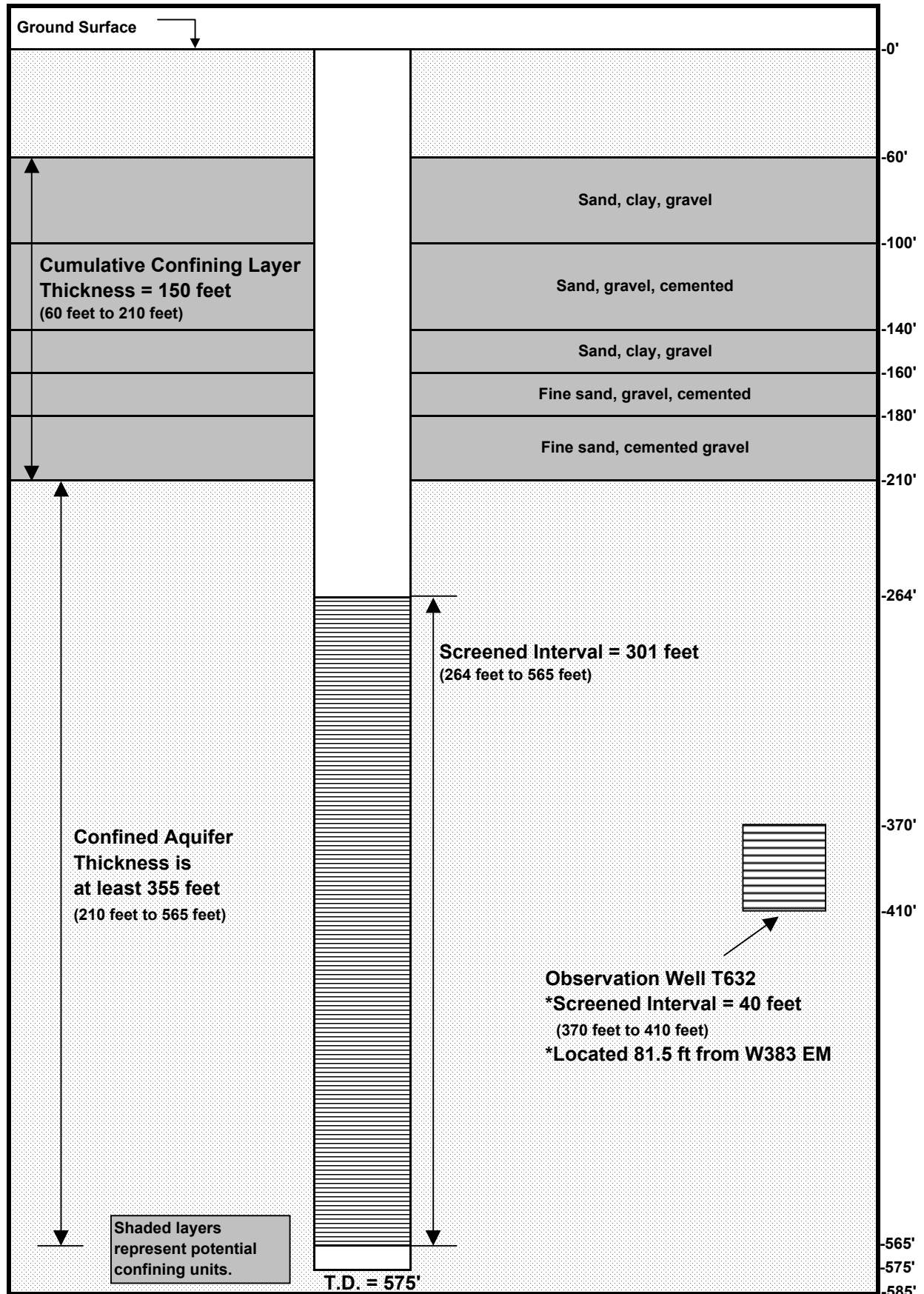


Figure B-8
Schematic Representation of Well W384 EM (Independence-Oak)

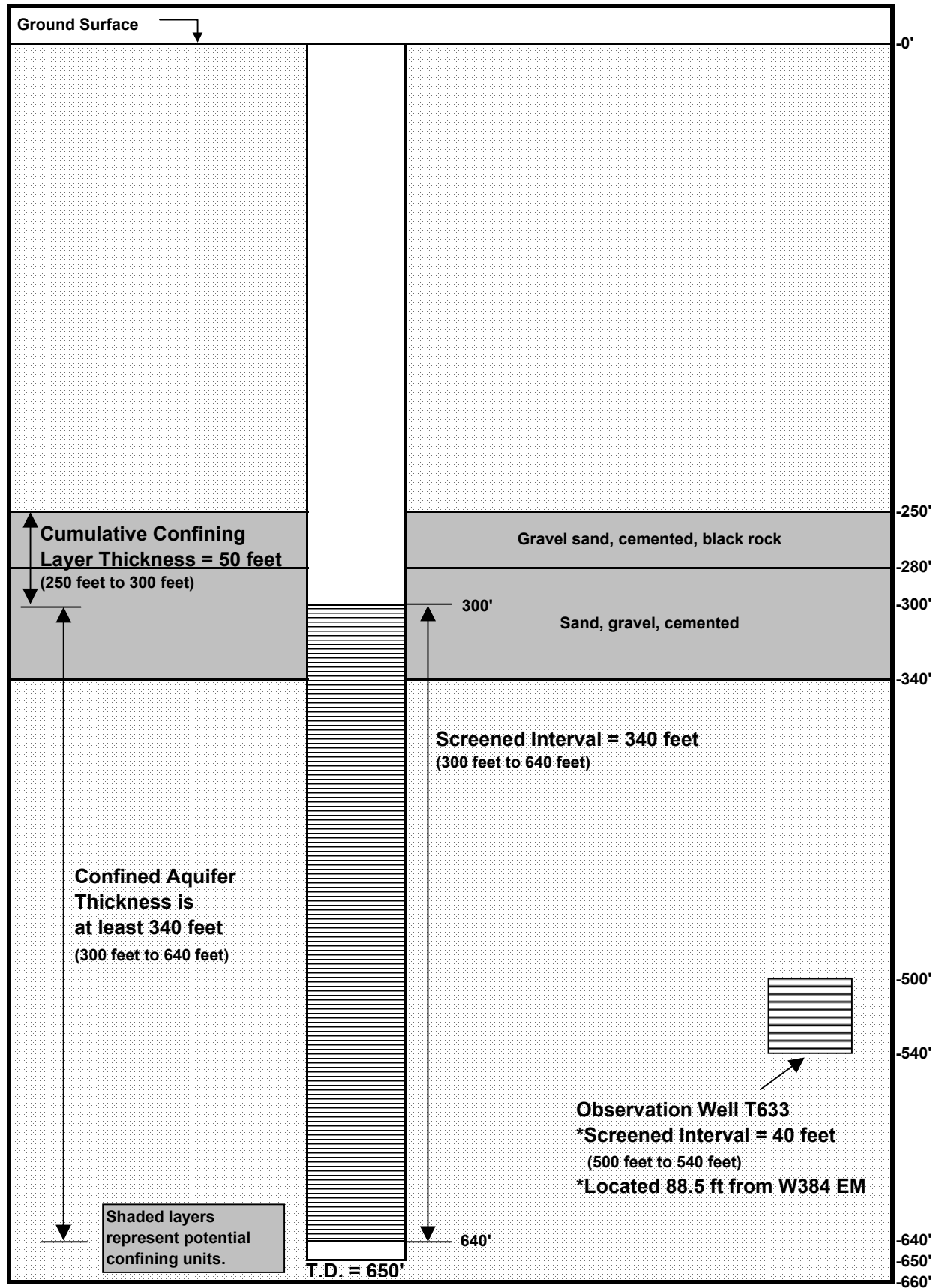
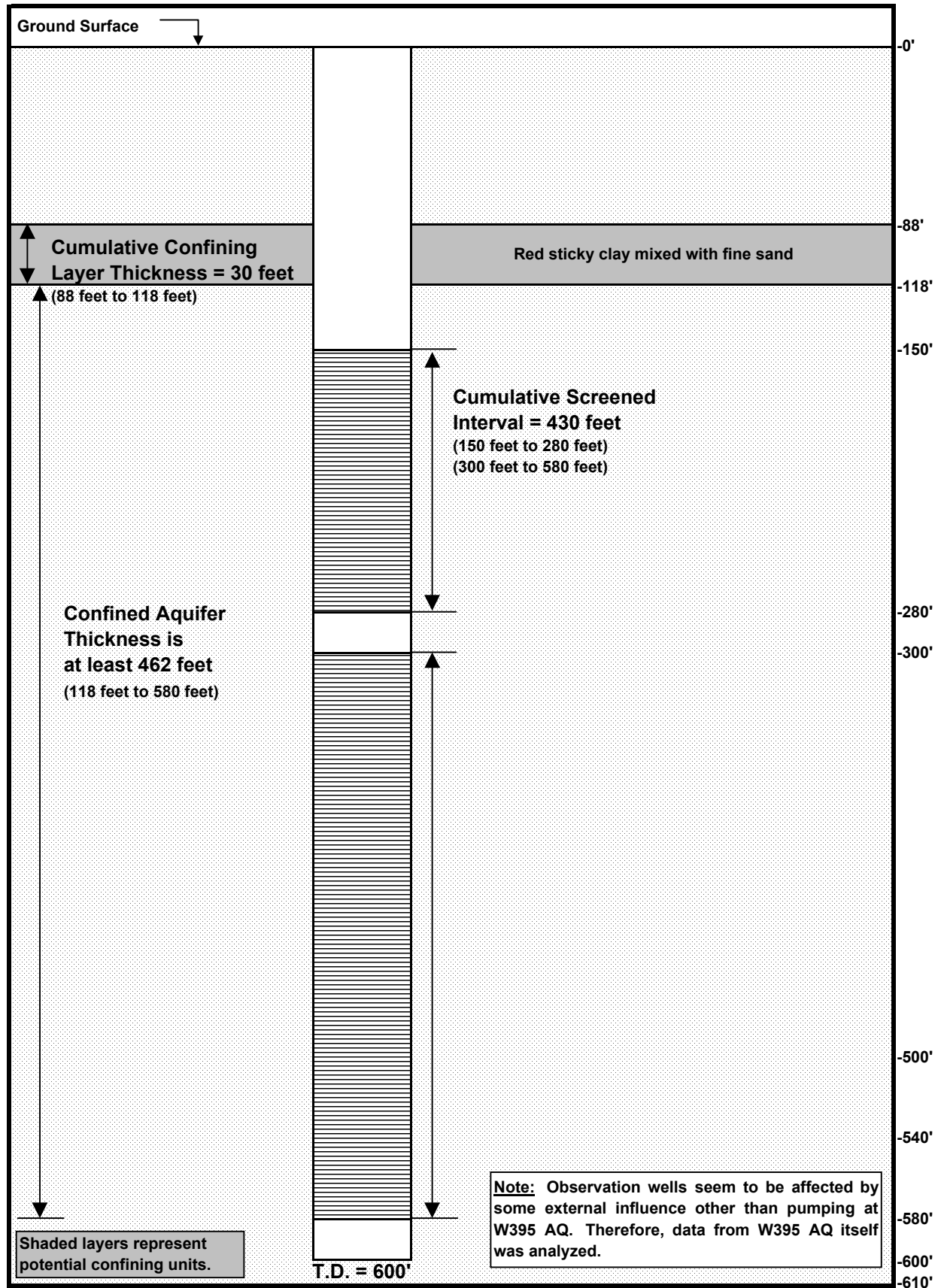


Figure B-9
Schematic Representation of Well W395 AQ (Symmes-Shepherd)



Appendix C
Drawdown vs. Time Data from the Nine Data Sets
Selected for Aquifer Test Analysis

Table C-1
Drawdown vs. Time Data for the Aquifer Test Conducted at
Well W387 EM (Laws)

| Pumping Well Data | |
|-------------------|---------------|
| Well W387 | |
| Time (min) | Drawdown (ft) |
| 8 | 77.83 |
| 14 | 80.53 |
| 16 | 82.36 |
| 31 | 86.13 |
| 35 | 86.96 |
| 40 | 87.74 |
| 45 | 88.37 |
| 50 | 89.24 |
| 60 | 90.04 |
| 70 | 91.97 |
| 82 | 92.74 |
| 90 | 93.04 |
| 100 | 93.55 |
| 110 | 93.78 |
| 120 | 94.22 |
| 140 | 95.79 |
| 160 | 96.37 |
| 180 | 96.73 |
| 200 | 98.66 |
| 220 | 99.18 |
| 240 | 99.44 |
| 290 | 100.05 |
| 330 | 99.97 |
| 360 | 99.88 |
| 390 | 102.09 |
| 420 | 102.02 |
| 480 | 101.92 |
| 540 | 101.99 |
| 600 | 101.89 |
| 660 | 102.79 |
| 720 | 102.39 |
| 780 | 102.39 |
| 840 | 103.39 |
| 900 | 103.39 |
| 960 | 103.39 |
| 1020 | 103.39 |
| 1080 | 102.89 |
| 1140 | 102.89 |
| 1200 | 103.89 |
| 1260 | 103.39 |
| 1290 | 103.69 |
| 1310 | 103.99 |
| 1319 | 103.69 |

*Only bolded measurements
were analyzed

Table C-2
Drawdown vs. Time Data for the Aquifer Test Conducted at
Well W398 AQ (Laws)

| Pumping Well Data | |
|-------------------|---------------|
| Well W398 | |
| Time (min) | Drawdown (ft) |
| 1 | 48.36 |
| 2 | 48.36 |
| 3 | 48.93 |
| 4 | 50.09 |
| 5 | 50.67 |
| 6 | 51.24 |
| 7 | 52.40 |
| 8 | 52.40 |
| 9 | 53.55 |
| 10 | 53.55 |
| 12 | 54.13 |
| 14 | 54.71 |
| 16 | 54.71 |
| 18 | 55.86 |
| 20 | 56.44 |
| 22 | 56.44 |
| 24 | 57.02 |
| 26 | 57.02 |
| 28 | 57.02 |
| 30 | 57.02 |
| 35 | 58.17 |
| 40 | 58.17 |
| 45 | 58.17 |
| 50 | 58.75 |
| 55 | 59.33 |
| 60 | 59.33 |
| 70 | 60.48 |
| 80 | 60.48 |
| 90 | 60.48 |
| 100 | 60.48 |
| 130 | 61.64 |
| 160 | 61.64 |
| 220 | 62.80 |
| 280 | 64.13 |
| 340 | 64.70 |
| 400 | 65.28 |
| 500 | 66.44 |
| 600 | 67.59 |
| 800 | 68.57 |
| 1000 | 68.57 |
| 1140 | 68.57 |

*Only bolded measurements
were analyzed

Table C-3
Drawdown vs. Time Data for the Aquifer Test Conducted at
Well V014GA (Big Pine)

| Pumping Well Data | | | |
|-------------------|---------------|-------------------------|---------------|
| Well V014GA | | Well V014GA (continued) | |
| Time (min) | Drawdown (ft) | Time (min) | Drawdown (ft) |
| 1 | 2.87 | 455 | 9.81 |
| 2 | 3.00 | 1430 | 10.25 |
| 3 | 3.56 | 1900 | 10.21 |
| 4 | 3.44 | 2950 | 10.41 |
| 5 | 3.50 | 3330 | 10.35 |
| 6 | 3.58 | 4318 | 10.55 |
| 7 | 3.62 | | |
| 8 | 3.67 | | |
| 9 | 3.73 | | |
| 10 | 3.76 | | |
| 12 | 3.81 | | |
| 14 | 3.85 | | |
| 16 | 3.90 | | |
| 18 | 3.93 | | |
| 20 | 3.98 | | |
| 25 | 4.04 | | |
| 30 | 4.14 | | |
| 35 | 4.14 | | |
| 40 | 4.19 | | |
| 45 | 4.22 | | |
| 50 | 4.24 | | |
| 55 | 4.27 | | |
| 60 | 4.30 | | |
| 70 | 4.32 | | |
| 80 | 4.37 | | |
| 90 | 4.41 | | |
| 100 | 4.44 | | |
| 110 | 4.47 | | |
| 120 | 4.48 | | |
| 135 | 4.50 | | |
| 150 | 4.52 | | |
| 165 | 4.54 | | |
| 180 | 4.56 | | |
| 195 | 4.57 | | |
| 210 | 4.58 | | |
| 225 | 4.59 | | |
| 240 | 4.60 | | |
| 241 | 8.02 | | |
| 242 | 8.28 | | |
| 243 | 8.39 | | |
| 244 | 8.51 | | |
| 245 | 8.59 | | |
| 246 | 8.66 | | |
| 247 | 8.69 | | |
| 248 | 8.74 | | |
| 249 | 8.79 | | |
| 250 | 8.83 | | |
| 255 | 8.95 | | |
| 260 | 9.06 | | |
| 265 | 9.13 | | |
| 270 | 9.20 | | |
| 275 | 9.26 | | |

*Only bolded measurements were analyzed.

Table C-4
Drawdown vs. Time Data for the Aquifer Test Conducted at
Well V016GA (Big Pine)

| Pumping Well Data | | | |
|-------------------|--------------|---------------------|--------------|
| Well V016GA | | Well V016GA (cont.) | |
| Time (min) | Drawdown(ft) | Time (min) | Drawdown(ft) |
| 16 | 16.69 | 1110 | 40.31 |
| 25 | 17.17 | 1115 | 40.53 |
| 30 | 17.11 | 1120 | 40.80 |
| 35 | 17.36 | 1125 | 41.00 |
| 40 | 17.42 | 1130 | 41.07 |
| 45 | 17.50 | 1135 | 41.20 |
| 50 | 17.56 | 1140 | 41.32 |
| 55 | 17.60 | 1150 | 41.07 |
| 60 | 17.64 | 1160 | 41.22 |
| 70 | 17.74 | 1170 | 41.38 |
| 80 | 17.80 | 1180 | 41.54 |
| 90 | 17.86 | 1190 | 41.75 |
| 100 | 17.93 | 1200 | 41.82 |
| 110 | 17.97 | 1215 | 42.11 |
| 120 | 18.00 | 1230 | 42.24 |
| 135 | 18.04 | 1245 | 42.43 |
| 150 | 18.08 | 1260 | 42.55 |
| 151 | 3.21 | 2637 | 43.41 |
| 152 | 2.40 | 5585 | 45.04 |
| 153 | 2.26 | 7035 | 45.16 |
| 154 | 1.78 | 8425 | 46.66 |
| 155 | 1.66 | 9900 | 45.53 |
| 156 | 1.54 | 12780 | 46.32 |
| 157 | 1.42 | 14220 | 46.11 |
| 158 | 1.36 | 15660 | 46.63 |
| 159 | 1.33 | | |
| 160 | 1.27 | | |
| 165 | 1.10 | | |
| 170 | 0.87 | | |
| 175 | 0.82 | | |
| 180 | 0.74 | | |
| 185 | 0.64 | | |
| 190 | 0.58 | | |
| 195 | 0.52 | | |
| 200 | 0.47 | | |
| 205 | 0.43 | | |
| 210 | 0.40 | | |
| 220 | 0.35 | | |
| 230 | 0.27 | | |
| 240 | 0.22 | | |
| 1075 | 0.02 | | |
| 1080 | 0.00 | | |
| 1087 | 38.17 | | |
| 1088 | 38.37 | | |
| 1089 | 38.55 | | |
| 1090 | 38.70 | | |
| 1092 | 38.98 | | |
| 1094 | 39.23 | | |
| 1096 | 39.43 | | |
| 1098 | 39.59 | | |
| 1100 | 39.37 | | |
| 1105 | 40.07 | | |

*Only bolded measurements were analyzed.

Table C-5
Drawdown vs. Time Data for the Aquifer Test Conducted at
Well W379 EM (Big Pine)

| Observation Well Data | | Pumping Well Data (not analyzed) | |
|-----------------------|---------------|----------------------------------|---------------|
| Well T627 | | Well W379 | |
| Time (min) | Drawdown (ft) | Time (min) | Drawdown (ft) |
| 1 | 7.40 | 1 | 24.90 |
| 2 | 8.62 | 2 | 27.30 |
| 3 | 10.68 | 3 | 29.60 |
| 4 | 11.83 | 4.3 | 31.70 |
| 5 | 12.65 | 5 | 32.50 |
| 6 | 13.32 | 6.5 | 32.90 |
| 7 | 13.64 | 7 | 33.20 |
| 8 | 14.17 | 8 | 33.95 |
| 9 | 14.59 | 9.5 | 34.18 |
| 10 | 15.02 | 10.5 | 35.00 |
| 15 | 16.51 | 12 | 35.60 |
| 20 | 17.41 | 14 | 36.16 |
| 25 | 18.10 | 16 | 36.71 |
| 30 | 18.69 | 18 | 37.15 |
| 40 | 19.82 | 20.5 | 36.90 |
| 50 | 20.89 | 22 | 37.12 |
| 60 | 21.33 | 24 | 37.49 |
| 70 | 21.94 | 26 | 37.78 |
| 80 | 22.39 | 28 | 38.10 |
| 90 | 22.78 | 30 | 38.20 |
| 105 | 23.41 | 35 | 38.90 |
| 120 | 23.77 | 40 | 39.40 |
| 135 | 24.18 | 45 | 39.95 |
| 150 | 24.61 | 50 | 40.24 |
| 165 | 24.90 | 56 | 40.70 |
| 180 | 25.31 | 60 | 40.60 |
| 210 | 26.67 | 70 | 41.45 |
| 240 | 27.26 | 84 | 42.05 |
| 270 | 27.69 | 90 | 42.20 |
| 300 | 28.05 | 92 | 42.37 |
| 330 | 28.37 | 100 | 42.60 |
| 360 | 28.66 | 110 | 42.87 |
| 390 | 29.04 | 120 | 42.95 |
| 420 | 29.24 | 130 | 43.28 |
| | | 142 | 43.53 |
| | | 150 | 43.74 |
| | | 160 | 44.02 |
| | | 170 | 44.27 |
| | | 181 | 44.33 |
| | | 193 | 46.42 |
| | | 200 | 46.46 |
| | | 211 | 46.95 |
| | | 220 | 47.05 |
| | | 240 | 47.38 |
| | | 250 | 47.60 |
| | | 260 | 47.60 |
| | | 271 | 47.85 |
| | | 285 | 48.00 |
| | | 300 | 48.14 |
| | | 315 | 48.18 |
| | | 330 | 48.27 |
| | | 360 | 48.62 |
| | | 380 | 48.86 |
| | | 420 | 49.00 |

*Only bolded measurements were analyzed.

Table C-6
Drawdown vs. Time Data for the Aquifer Test Conducted at
Well W389 EM (Big Pine)

| Observation Well Data | | Pumping Well Data (not analyzed) | | | |
|-----------------------|---------------|----------------------------------|--------------|-------------------|--------------|
| Well T736 | | Well W389 | | Well W389 (cont.) | |
| Time (min) | Drawdown (ft) | Time (min) | Drawdown(ft) | Time (min) | Drawdown(ft) |
| 1 | 6.07 | 1 | 29.60 | 720 | 59.70 |
| 2 | 9.23 | 2 | 36.30 | 780 | 60.10 |
| 3 | 11.04 | 3 | 38.80 | 840 | 60.50 |
| 4 | 12.37 | 4 | 40.30 | 900 | 60.50 |
| 5 | 13.42 | 5 | 41.50 | 960 | 60.70 |
| 6 | 14.22 | 6 | 42.20 | 1020 | 60.70 |
| 7 | 14.62 | 7 | 42.10 | 1080 | 60.80 |
| 8 | 15.09 | 8 | 42.70 | 1140 | 61.50 |
| 9 | 15.50 | 9 | 43.10 | 1200 | 61.70 |
| 10 | 15.75 | 10 | 43.30 | 1260 | 61.90 |
| 12 | 16.37 | 12 | 43.90 | | |
| 14 | 16.97 | 14 | 44.40 | | |
| 16 | 17.48 | 16 | 44.90 | | |
| 18 | 17.94 | 18 | 45.40 | | |
| 20 | 18.37 | 20 | 45.80 | | |
| 25 | 19.25 | 25 | 46.60 | | |
| 30 | 19.97 | 30 | 47.40 | | |
| 43 | 21.40 | 35 | 48.00 | | |
| 45 | 21.56 | 40.5 | 48.50 | | |
| 50 | 21.93 | 45 | 48.80 | | |
| 55 | 22.31 | 50 | 49.10 | | |
| 60 | 22.63 | 55 | 49.50 | | |
| 66 | 22.97 | 60 | 49.70 | | |
| 70 | 23.23 | 65 | 50.00 | | |
| 75 | 22.80 | 70 | 50.30 | | |
| 80 | 23.76 | 75 | 50.70 | | |
| 85 | 24.04 | 80 | 51.00 | | |
| 90 | 24.20 | 85 | 51.30 | | |
| 97 | 24.45 | 90 | 51.30 | | |
| 100 | 24.54 | 95 | 51.40 | | |
| 105 | 24.72 | 100 | 51.50 | | |
| 110 | 24.86 | 110 | 51.70 | | |
| 115 | 25.04 | 120 | 52.10 | | |
| 120 | 25.20 | 135 | 52.60 | | |
| 135 | 25.66 | 150 | 52.70 | | |
| 150 | 25.97 | 165 | 52.80 | | |
| 165 | 26.24 | 180 | 53.00 | | |
| 180 | 26.47 | 195 | 53.30 | | |
| 195 | 26.79 | 210 | 53.60 | | |
| 210 | 27.07 | 240 | 56.20 | | |
| 225 | 28.01 | 255 | 56.30 | | |
| 240 | 28.40 | 270 | 56.60 | | |
| 255 | 28.62 | 289 | 56.60 | | |
| 270 | 28.85 | 300 | 56.80 | | |
| 285 | 29.99 | 315 | 56.90 | | |
| 300 | 29.18 | 330 | 57.10 | | |
| 330 | 29.52 | 345 | 57.40 | | |
| 360 | 29.85 | 360 | 57.30 | | |
| 390 | 30.13 | 390 | 57.60 | | |
| 420 | 30.42 | 420 | 57.90 | | |
| 1203 | 34.09 | 480 | 58.40 | | |
| 1215 | 34.10 | 540 | 58.70 | | |
| 1230 | 34.16 | 600 | 58.80 | | |
| 1245 | 34.26 | 660 | 59.30 | | |

*Only bolded measurements were analyzed

Table C-7
Drawdown vs. Time Data for the Aquifer Test Conducted at
Well W383 EM (Independence-Oak)

| Observation Well Data | | Pumping Well Data (not analyzed) | | | |
|-----------------------|---------------|----------------------------------|---------------|-----------------------|---------------|
| Well T632 | | Well W383 | | Well W383 (continued) | |
| Time (min) | Drawdown (ft) | Time (min) | Drawdown (ft) | Time (min) | Drawdown (ft) |
| 1 | 3.69 | 3 | 96.41 | 690 | 143.26 |
| 2 | 7.21 | 4 | 97.61 | 720 | 143.91 |
| 3 | 9.44 | 6 | 125.16 | 750 | 144.21 |
| 4 | 11.04 | 7 | 126.66 | 780 | 144.48 |
| 5 | 12.28 | 8 | 127.51 | 810 | 144.54 |
| 6 | 13.29 | 9 | 128.41 | 840 | 144.18 |
| 7 | 14.19 | 10 | 129.31 | 870 | 144.76 |
| 8 | 14.91 | 12 | 130.96 | 900 | 144.41 |
| 9 | 15.58 | 14 | 132.46 | 930 | 143.75 |
| 10 | 16.17 | 16 | 132.71 | 960 | 143.84 |
| 12 | 17.15 | 18 | 133.11 | 990 | 144.19 |
| 14 | 17.95 | 20 | 133.71 | 1020 | 144.34 |
| 16 | 18.66 | 22 | 134.11 | 1050 | 144.48 |
| 18 | 19.24 | 24 | 134.61 | 1080 | 144.56 |
| 20 | 19.73 | 26 | 134.96 | 1110 | 144.43 |
| 22 | 20.16 | 28 | 135.09 | 1140 | 145.26 |
| 24 | 20.53 | 30 | 135.46 | 1170 | 145.43 |
| 26 | 20.94 | 35 | 135.56 | 1200 | 145.54 |
| 28 | 21.29 | 40 | 136.21 | 1230 | 145.43 |
| 30 | 21.59 | 45 | 136.51 | 1260 | 146.09 |
| 35 | 22.24 | 50 | 136.76 | 1290 | 145.13 |
| 40 | 22.78 | 55 | 136.66 | | |
| 45 | 23.26 | 60 | 136.71 | | |
| 50 | 23.63 | 65 | 136.85 | | |
| 55 | 23.94 | 70 | 137.31 | | |
| 60 | 24.21 | 75 | 137.26 | | |
| 65 | 24.50 | 80 | 137.42 | | |
| 70 | 24.75 | 85 | 137.39 | | |
| 75 | 25.00 | 90 | 137.55 | | |
| 80 | 25.18 | 100 | 138.31 | | |
| 85 | 25.36 | 110 | 138.88 | | |
| 90 | 25.53 | 120 | 138.84 | | |
| 100 | 25.78 | 130 | 139.46 | | |
| 110 | 26.13 | 140 | 139.39 | | |
| 120 | 26.31 | 150 | 139.66 | | |
| 130 | 26.54 | 160 | 139.94 | | |
| 140 | 26.71 | 170 | 139.71 | | |
| 150 | 26.92 | 180 | 139.66 | | |
| 161 | 27.06 | 200 | 139.56 | | |
| 170 | 27.18 | 220 | 138.91 | | |
| 180 | 27.28 | 240 | 138.62 | | |
| 200 | 27.42 | 260 | 142.62 | | |
| 220 | 27.50 | 280 | 142.51 | | |
| 240 | 27.57 | 300 | 142.35 | | |
| 260 | 28.15 | 330 | 141.97 | | |
| 280 | 28.29 | 360 | 141.89 | | |
| 300 | 28.41 | 390 | 141.66 | | |
| 330 | 28.50 | 420 | 141.59 | | |
| 360 | 28.58 | 450 | 140.99 | | |
| 390 | 28.61 | 480 | 143.53 | | |
| 420 | 28.63 | 510 | 143.01 | | |
| 450 | 28.65 | 540 | 142.60 | | |
| 480 | 29.07 | 570 | 142.83 | | |
| 1275 | 29.99 | 600 | 143.32 | | |
| 1290 | 29.94 | 630 | 143.47 | | |
| 1320 | 29.89 | 660 | 142.56 | | |

*Only bolded measurements were analyzed.

Table C-8
Drawdown vs. Time Data for the Aquifer Test Conducted at
Well W384 EM (Independence-Oak)

| Observation Well Data | | | | Pumping Well Data (not analyzed) | | | |
|-----------------------|---------------|-----------------------|---------------|----------------------------------|---------------|-----------------------|---------------|
| Well T633 | | Well T633 (continued) | | Well W384 | | Well W384 (continued) | |
| Time (min) | Drawdown (ft) | Time (min) | Drawdown (ft) | Time (min) | Drawdown (ft) | Time (min) | Drawdown (ft) |
| 1 | 1.92 | 450 | 20.55 | 5 | 68.33 | 630 | 78.30 |
| 2 | 4.49 | 465 | 20.54 | 6 | 69.88 | 660 | 78.22 |
| 3 | 6.24 | 480 | 20.56 | 7 | 71.13 | 690 | 78.07 |
| 4 | 7.48 | 1290 | 22.33 | 8 | 71.33 | 720 | 78.20 |
| 5 | 8.43 | 1305 | 22.30 | 9 | 72.03 | 750 | 84.40 |
| 6 | 9.18 | 1320 | 22.31 | 10 | 72.63 | 780 | 84.64 |
| 7 | 9.80 | | | 12 | 73.53 | 810 | 81.68 |
| 8 | 10.34 | | | 14 | 74.18 | 840 | 81.31 |
| 9 | 10.79 | | | 16 | 74.73 | 870 | 81.24 |
| 10 | 11.16 | | | 18 | 75.23 | 900 | 81.21 |
| 12 | 11.82 | | | 20 | 75.53 | 930 | 81.32 |
| 14 | 12.37 | | | 25 | 75.98 | 960 | 83.95 |
| 16 | 12.84 | | | 30 | 76.53 | 990 | 85.14 |
| 18 | 13.23 | | | 35 | 76.63 | 1020 | 85.60 |
| 20 | 13.56 | | | 40 | 77.03 | 1050 | 85.12 |
| 22 | 13.87 | | | 45 | 77.18 | 1080 | 83.13 |
| 24 | 14.14 | | | 50 | 77.34 | 1110 | 83.32 |
| 26 | 14.41 | | | 55 | 77.58 | 1140 | 83.40 |
| 28 | 14.63 | | | 60 | 77.83 | 1170 | 83.34 |
| 30 | 14.82 | | | 65 | 77.83 | 1200 | 83.27 |
| 35 | 15.24 | | | 70 | 78.10 | 1230 | 83.15 |
| 40 | 15.63 | | | 75 | 78.08 | 1260 | 83.10 |
| 45 | 15.94 | | | 85 | 78.13 | 1290 | 83.30 |
| 50 | 16.23 | | | 90 | 78.18 | 1320 | 83.36 |
| 55 | 16.53 | | | 100 | 78.33 | | |
| 60 | 16.73 | | | 110 | 78.38 | | |
| 65 | 16.92 | | | 120 | 78.53 | | |
| 70 | 17.15 | | | 130 | 78.66 | | |
| 75 | 17.33 | | | 140 | 79.48 | | |
| 80 | 17.46 | | | 150 | 79.33 | | |
| 85 | 17.62 | | | 160 | 79.83 | | |
| 90 | 17.73 | | | 170 | 80.10 | | |
| 100 | 17.97 | | | 180 | 79.58 | | |
| 110 | 18.21 | | | 190 | 79.78 | | |
| 120 | 18.43 | | | 200 | 79.75 | | |
| 130 | 18.63 | | | 210 | 79.40 | | |
| 140 | 18.85 | | | 225 | 79.53 | | |
| 150 | 19.03 | | | 240 | 79.13 | | |
| 160 | 19.21 | | | 260 | 79.58 | | |
| 170 | 19.39 | | | 280 | 80.13 | | |
| 180 | 19.47 | | | 300 | 79.73 | | |
| 190 | 19.59 | | | 330 | 79.63 | | |
| 200 | 19.67 | | | 360 | 79.08 | | |
| 210 | 19.70 | | | 390 | 74.88 | | |
| 225 | 19.82 | | | 405 | 74.18 | | |
| 240 | 19.89 | | | 420 | 80.13 | | |
| 260 | 20.06 | | | 435 | 79.47 | | |
| 280 | 20.21 | | | 450 | 79.02 | | |
| 300 | 20.28 | | | 465 | 78.88 | | |
| 320 | 20.32 | | | 480 | 78.73 | | |
| 340 | 20.35 | | | 510 | 78.68 | | |
| 360 | 20.31 | | | 540 | 78.48 | | |
| 420 | 20.54 | | | 570 | 78.63 | | |
| 435 | 20.57 | | | 600 | 78.53 | | |

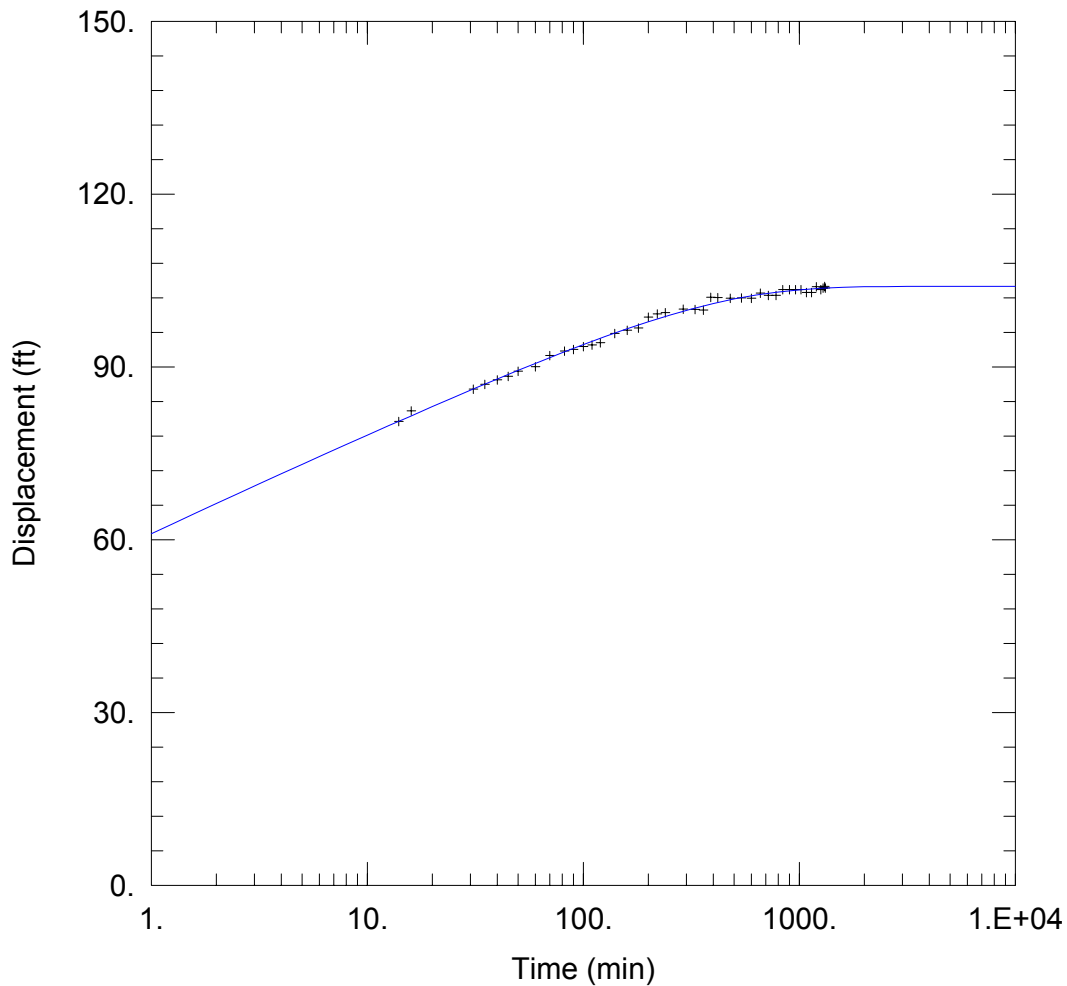
*Only bolded measurements were analyzed.

Table C-9
Drawdown vs. Time Data for the Aquifer Test Conducted at
Well W395 AQ (Symmes-Shepherd)

| Pumping Well Data | | | |
|-------------------|---------------|-------------------|---------------|
| Well W395 | | Well W395 (cont.) | |
| Time (min) | Drawdown (ft) | Time (min) | Drawdown (ft) |
| 2 | 16.64 | 1200 | 25.85 |
| 3 | 17.05 | 1260 | 25.77 |
| 4 | 17.55 | 1380 | 25.75 |
| 5 | 17.97 | 1560 | 25.75 |
| 6 | 18.35 | 1740 | 25.65 |
| 7 | 18.59 | 1800 | 25.91 |
| 8 | 18.75 | 1980 | 26.01 |
| 9 | 18.98 | 2160 | 26.38 |
| 10 | 19.29 | 2400 | 26.46 |
| 12 | 19.53 | 2640 | 26.59 |
| 14 | 19.78 | 2880 | 26.75 |
| 16 | 19.98 | 3000 | 26.67 |
| 18 | 20.22 | 3360 | 26.85 |
| 20 | 20.46 | 3720 | 26.94 |
| 25 | 20.78 | 4080 | 27.07 |
| 30 | 20.81 | 4320 | 26.95 |
| 40 | 21.43 | 4440 | 26.95 |
| 45 | 21.50 | 4800 | 26.97 |
| 50 | 21.80 | 5160 | 27.02 |
| 55 | 21.94 | 5520 | 27.00 |
| 60 | 22.06 | 5755 | 27.04 |
| 70 | 22.07 | | |
| 80 | 22.36 | | |
| 90 | 22.64 | | |
| 100 | 22.76 | | |
| 110 | 22.75 | | |
| 120 | 22.87 | | |
| 130 | 22.90 | | |
| 140 | 22.99 | | |
| 150 | 23.12 | | |
| 160 | 23.27 | | |
| 170 | 23.26 | | |
| 180 | 23.40 | | |
| 195 | 23.45 | | |
| 210 | 23.48 | | |
| 240 | 23.64 | | |
| 270 | 24.15 | | |
| 300 | 24.25 | | |
| 360 | 24.50 | | |
| 420 | 24.73 | | |
| 480 | 25.06 | | |
| 540 | 25.06 | | |
| 600 | 25.15 | | |
| 660 | 25.27 | | |
| 720 | 25.35 | | |
| 780 | 25.46 | | |
| 840 | 25.50 | | |
| 900 | 25.54 | | |
| 960 | 25.76 | | |
| 1020 | 25.78 | | |
| 1080 | 25.58 | | |
| 1140 | 25.75 | | |

*Only bolded measurements were analyzed

Appendix D
AQTESOLV Output for
Well W387 EM (Laws)



WELL TEST ANALYSIS

Data Set: \...\387 EM_HJ.aqt

Date: 12/04/02

Time: 14:58:37

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W387 EM

Test Date: April 1987

WELL DATA

Pumping Wells

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W387 EM | 0 | 0 |

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| + W387 EM | 1.17 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush-Jacob

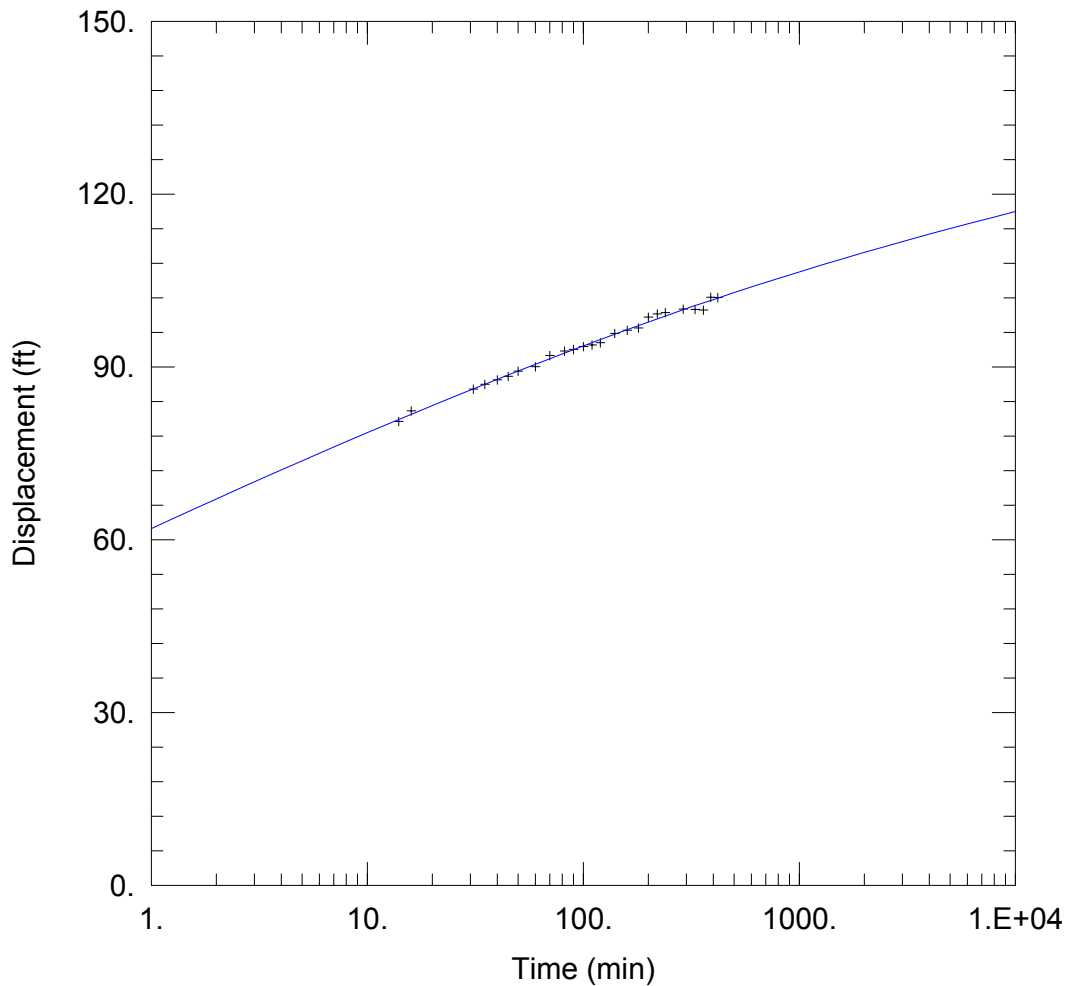
T = 5.298 ft²/min

S = 0.004245

r/B = 0.001337

Kz/Kr = 1.

b = 370. ft



WELL TEST ANALYSIS

Data Set: \\...\387 EM_H.aqt
 Date: 12/04/02

Time: 14:58:50

PROJECT INFORMATION

Company: MWH
 Client: LADWP
 Test Well: W387 EM
 Test Date: April 1987

WELL DATA

Pumping Wells

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W387 EM | 0 | 0 |

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| + W387 EM | 1.17 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush

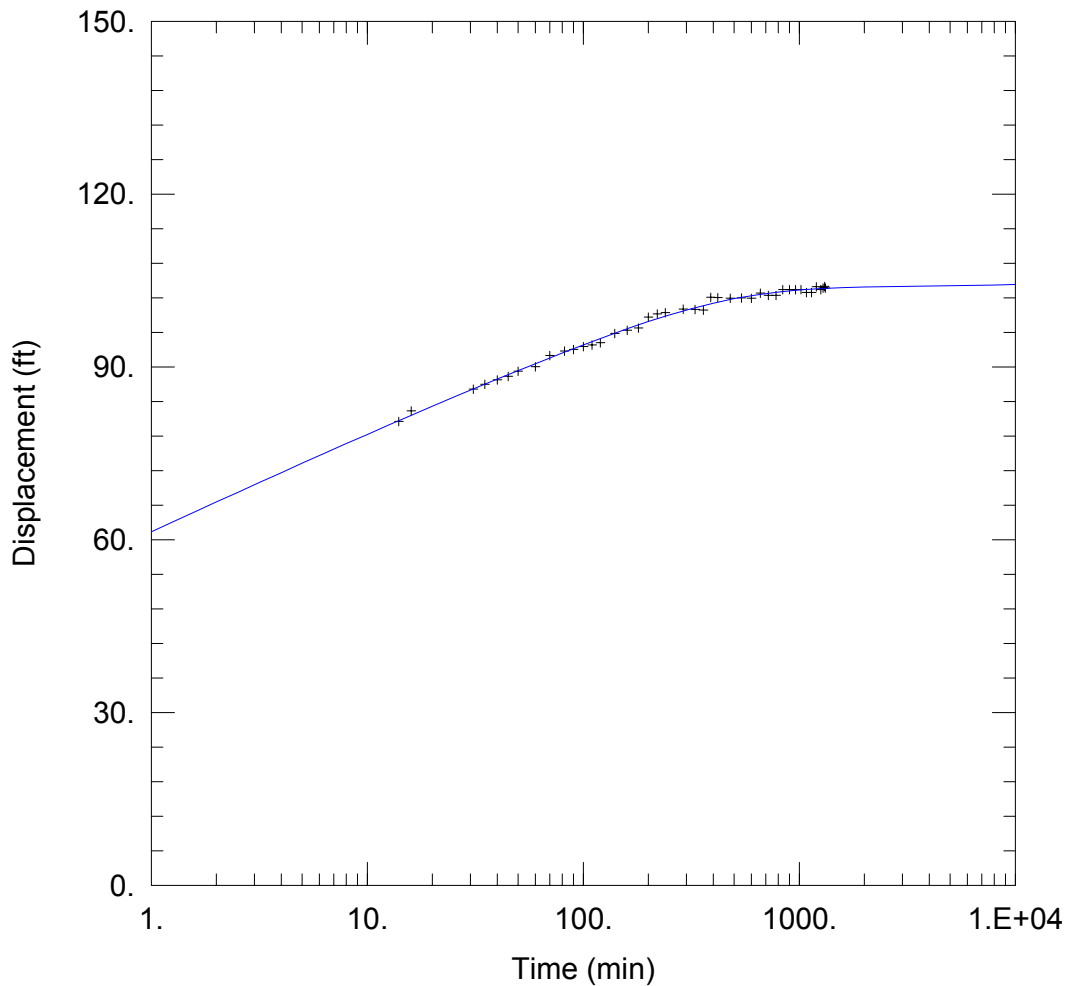
T = 5.273 ft²/min

S = 0.003745

β = 0.0002972

Kz/Kr = 1.

b = 370. ft



WELL TEST ANALYSIS

Data Set: \...\387 EM_NW.aqt
Date: 12/04/02

Time: 14:58:58

PROJECT INFORMATION

Company: MWH
Client: LADWP
Test Well: W387 EM
Test Date: April 1987

AQUIFER DATA

Saturated Thickness: 370 ft

Anisotropy Ratio (K_z/K_r): 1

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W387 EM | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| + W387 EM | 1.17 | 0 |

SOLUTION

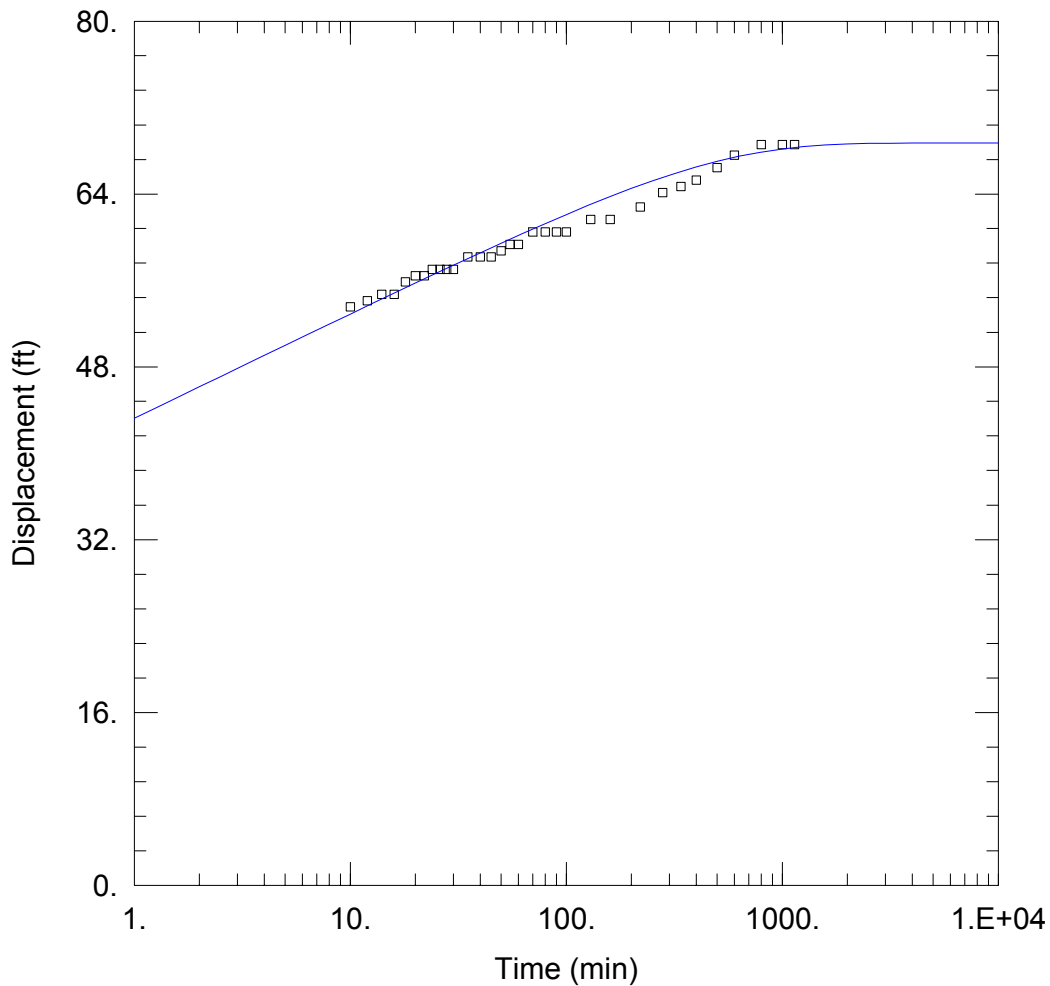
Aquifer Model: Leaky

Solution Method: Neuman-Witherspoon

$T = 4.976 \text{ ft}^2/\text{min}$
 $r/B = 0.001344$
 $T' = 1.243\text{E-}05 \text{ ft}^2/\text{min}$

$S = 0.002734$
 $\beta = 0.0003301$
 $S' = 1$

Appendix E
AQTESOLV Output for
Well W398 AQ (Laws)



WELL TEST ANALYSIS

Data Set: \...\398 AQ_HJ.aqt

Date: 11/14/02

Time: 18:06:59

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W398 AQ

Test Date: April 1991

WELL DATA

Pumping Wells

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W398 AQ | 0 | 0 |

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ W398 AQ | 1.17 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush-Jacob

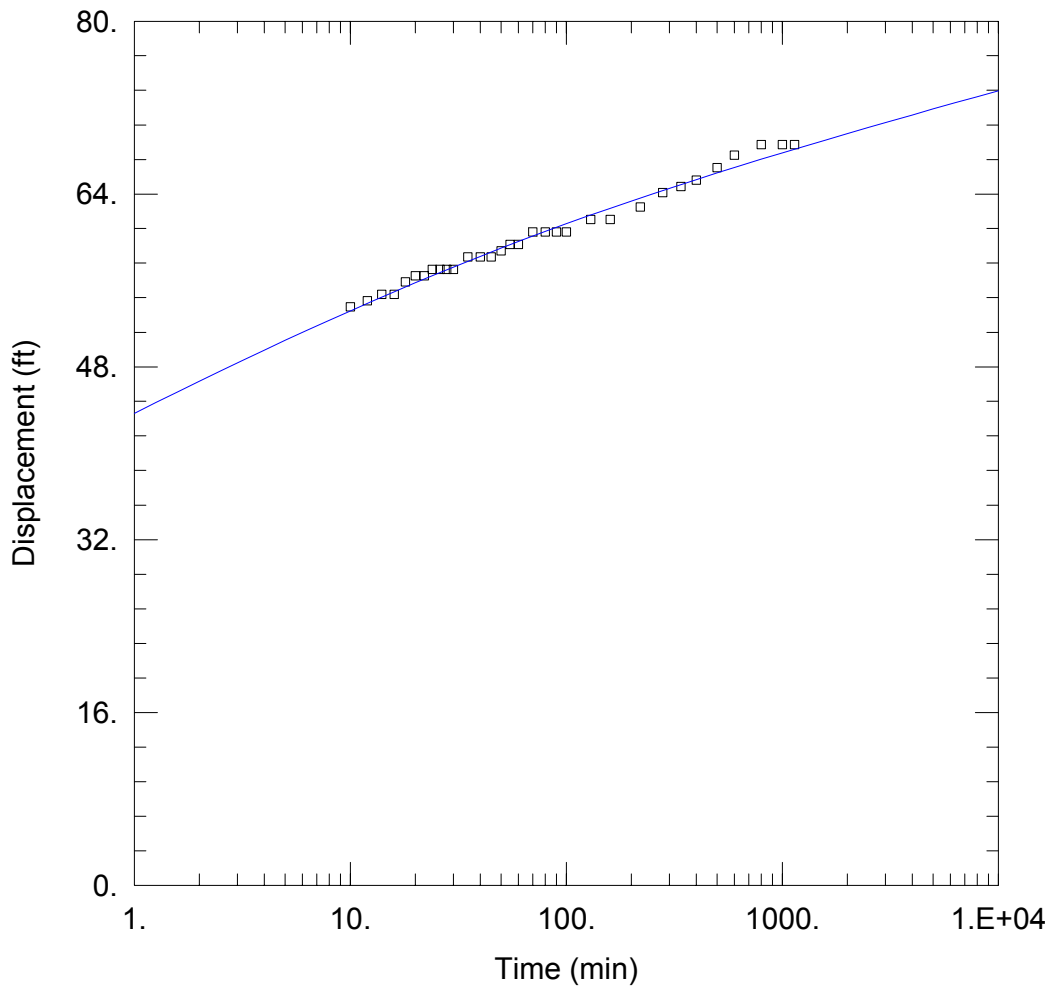
T = 7.575 ft²/min

S = 0.0005757

r/B = 0.0003715

Kz/Kr = 1.

b = 367. ft



WELL TEST ANALYSIS

Data Set: \\...\398 AQ_H.aqt

Date: 11/14/02

Time: 18:09:05

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W398 AQ

Test Date: April 1991

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W398 AQ | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ W398 AQ | 1.17 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush

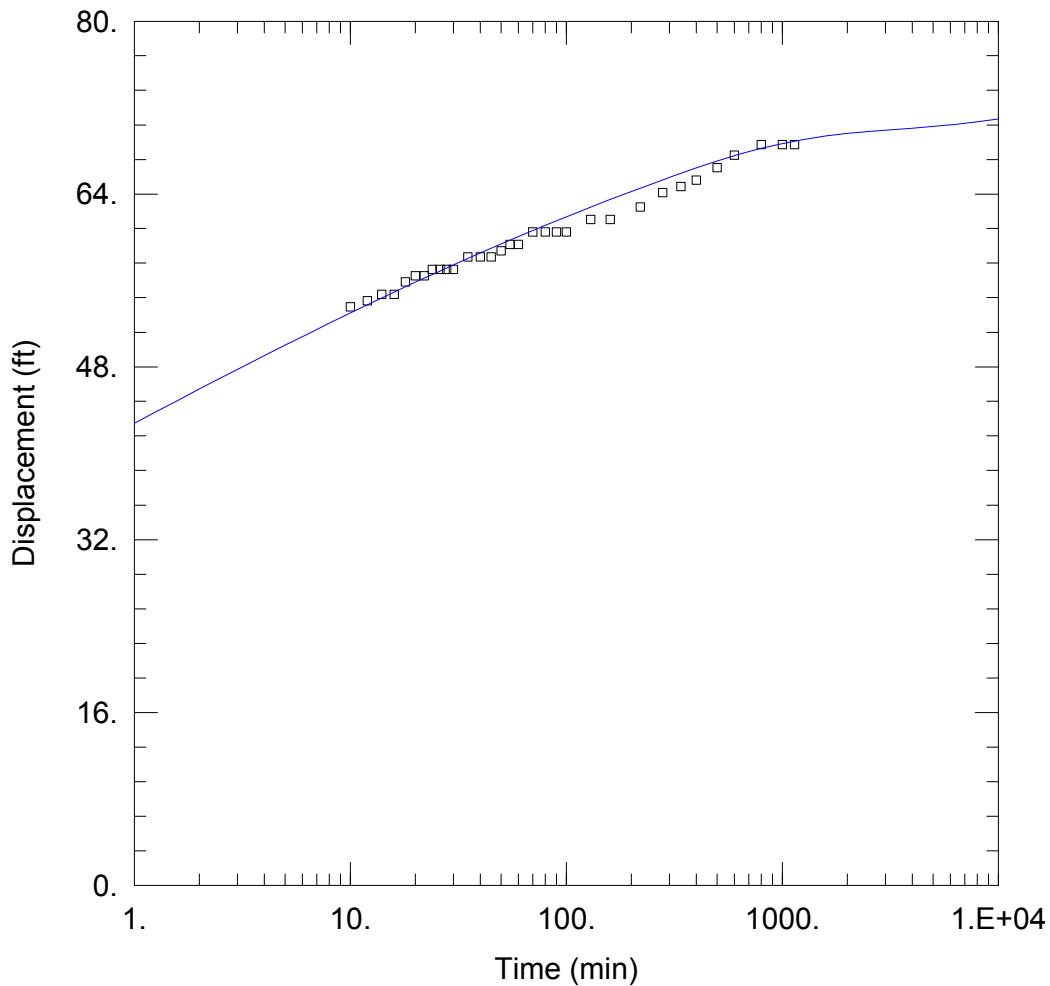
T = 6.908 ft²/min

S = 0.001019

β = 0.0004571

Kz/Kr = 1.

b = 367. ft



WELL TEST ANALYSIS

Data Set: \...\398 AQ_NW.aqt
Date: 11/14/02

Time: 18:14:56

PROJECT INFORMATION

Company: MWH
Client: LADWP
Test Well: W398 AQ
Test Date: April 1991

AQUIFER DATA

Saturated Thickness: 367 ft

Anisotropy Ratio (K_z/K_r): 1

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W398 AQ | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ W398 AQ | 1.17 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Neuman-Witherspoon

$T = 6.597 \text{ ft}^2/\text{min}$

$S = 0.001411$

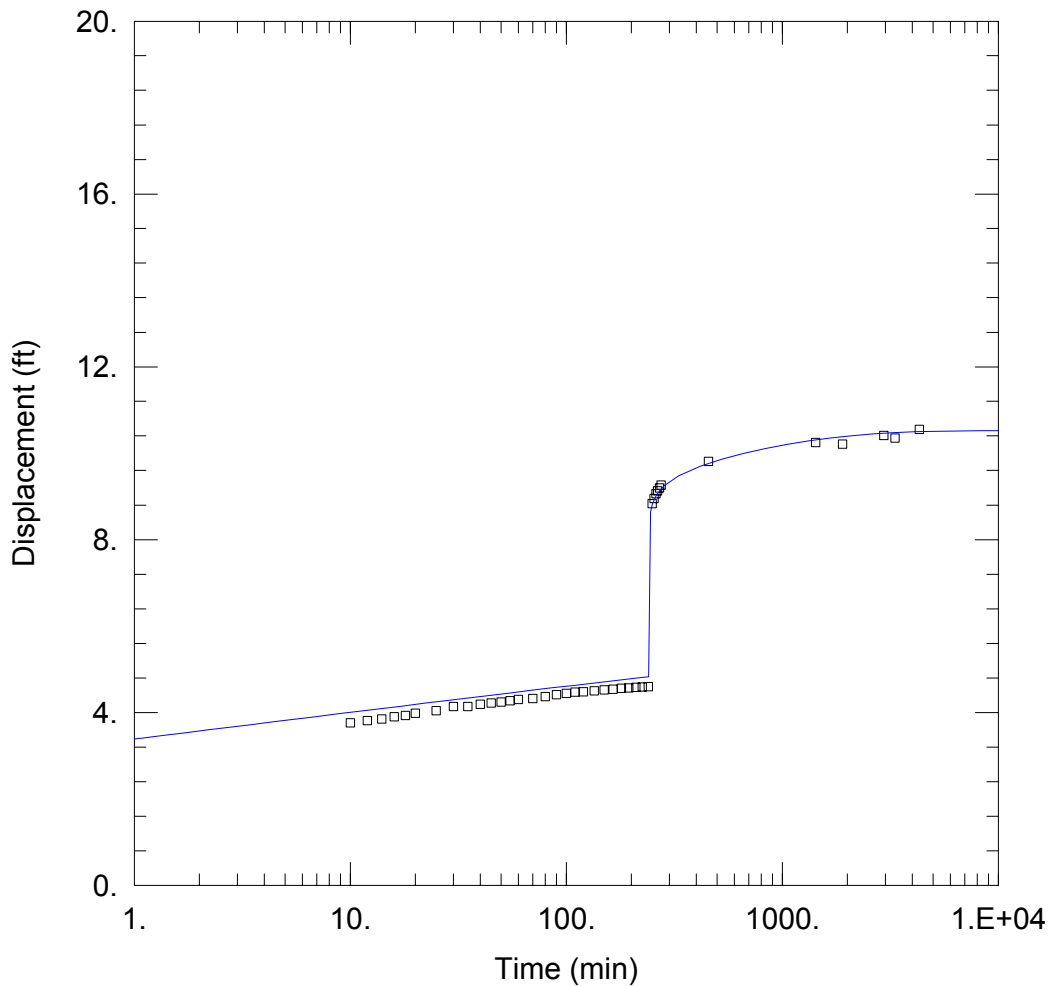
$r/B = 0.0008318$

$\beta = 0.0003981$

$T' = 0.1 \text{ ft}^2/\text{min}$

$S' = 0.1$

Appendix F
AQTESOLV Output for
Well V014GA (Big Pine)



WELL TEST ANALYSIS

Data Set: \...\V014GA_HJ.aqt

Date: 11/14/02

Time: 11:01:13

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: V014GA

Test Date: September 1984

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| V014GA | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ V014GA | 0.667 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush-Jacob

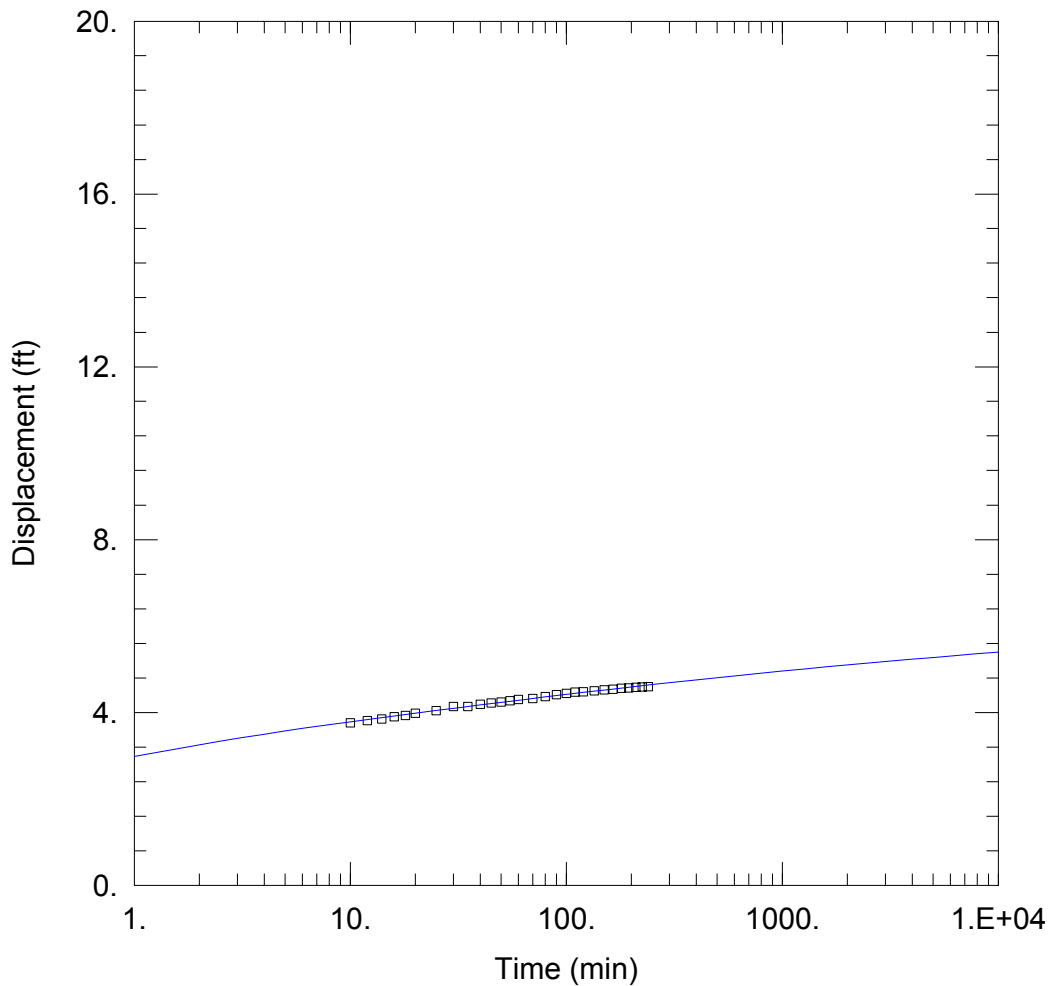
T = 3.767 ft²/min

S = 0.003406

r/B = 0.0004555

Kz/Kr = 1.

b = 65. ft



WELL TEST ANALYSIS

Data Set: \\...\V014GA_H.aqt

Date: 11/14/02

Time: 15:11:01

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: V014GA

Test Date: September 1984

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| V014GA | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ V014GA | 0.667 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush

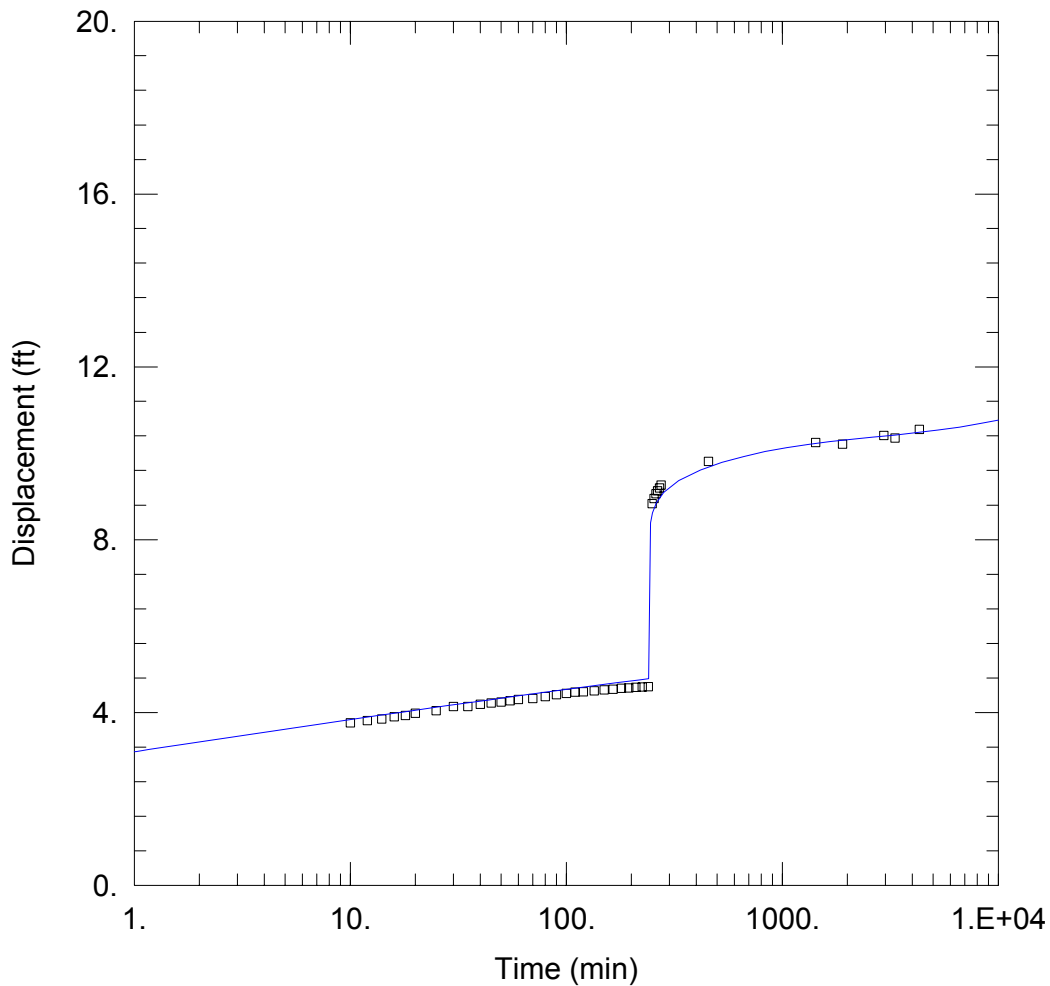
T = 3.36 ft²/min

S = 0.02747

β = 0.0005159

Kz/Kr = 1.

b = 65. ft



WELL TEST ANALYSIS

Data Set: \...V014GA_NW.aqt

Date: 11/14/02

Time: 13:16:50

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: V014GA

Test Date: September 1984

AQUIFER DATA

Saturated Thickness: 65 ft

Anisotropy Ratio (K_z/K_r): 1.

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| V014GA | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ V014GA | 0.667 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Neuman-Witherspoon

$T = 3.03 \text{ ft}^2/\text{min}$

$S = 0.001406$

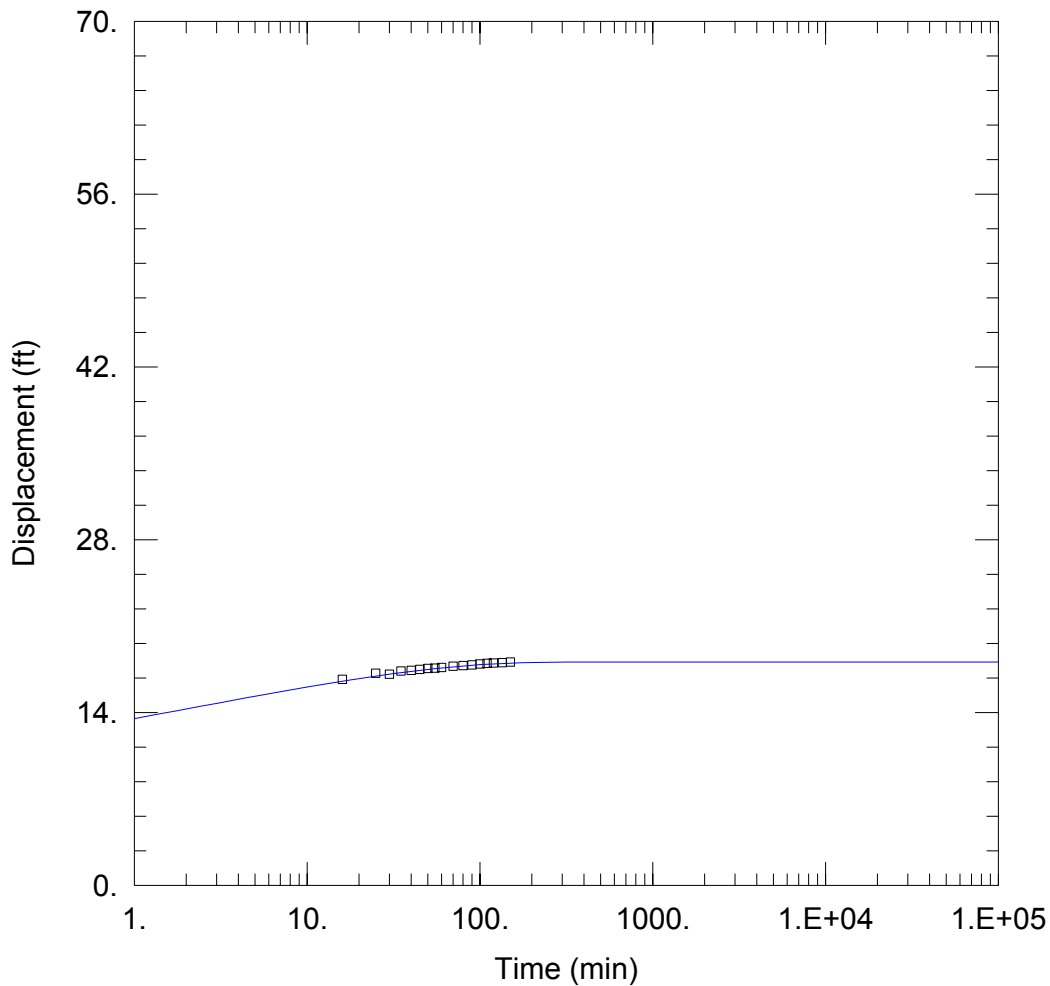
$r/B = 0.0004962$

$\beta = 0.0001114$

$T' = 0.0001049 \text{ ft}^2/\text{min}$

$S' = 0.01583$

Appendix G
AQTESOLV Output for
Well V016GA (Big Pine)



WELL TEST ANALYSIS

Data Set: \\...\V016GA_HJ.aqt

Date: 11/14/02

Time: 18:25:27

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: V016GA

Test Date: June 1985

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| V016GA | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ V016GA | 0.625 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush-Jacob

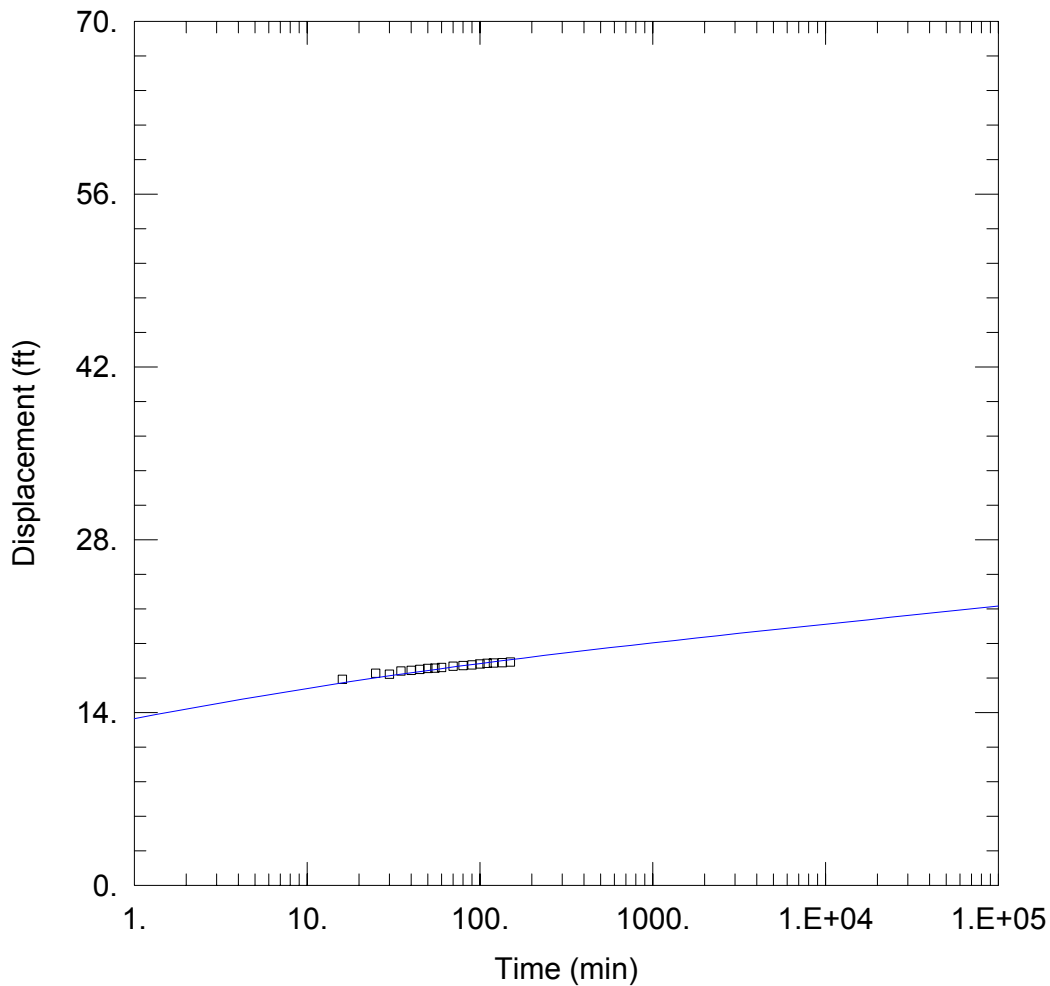
T = 2.383 ft²/min

S = 0.0003162

r/B = 0.0007413

Kz/Kr = 1.

b = 48. ft



WELL TEST ANALYSIS

Data Set: \...\V016GA_H.aqt

Date: 11/14/02

Time: 18:42:15

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: V016GA

Test Date: June 1985

WELL DATA

Pumping Wells

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| V016GA | 0 | 0 |

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ V016GA | 0.625 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush

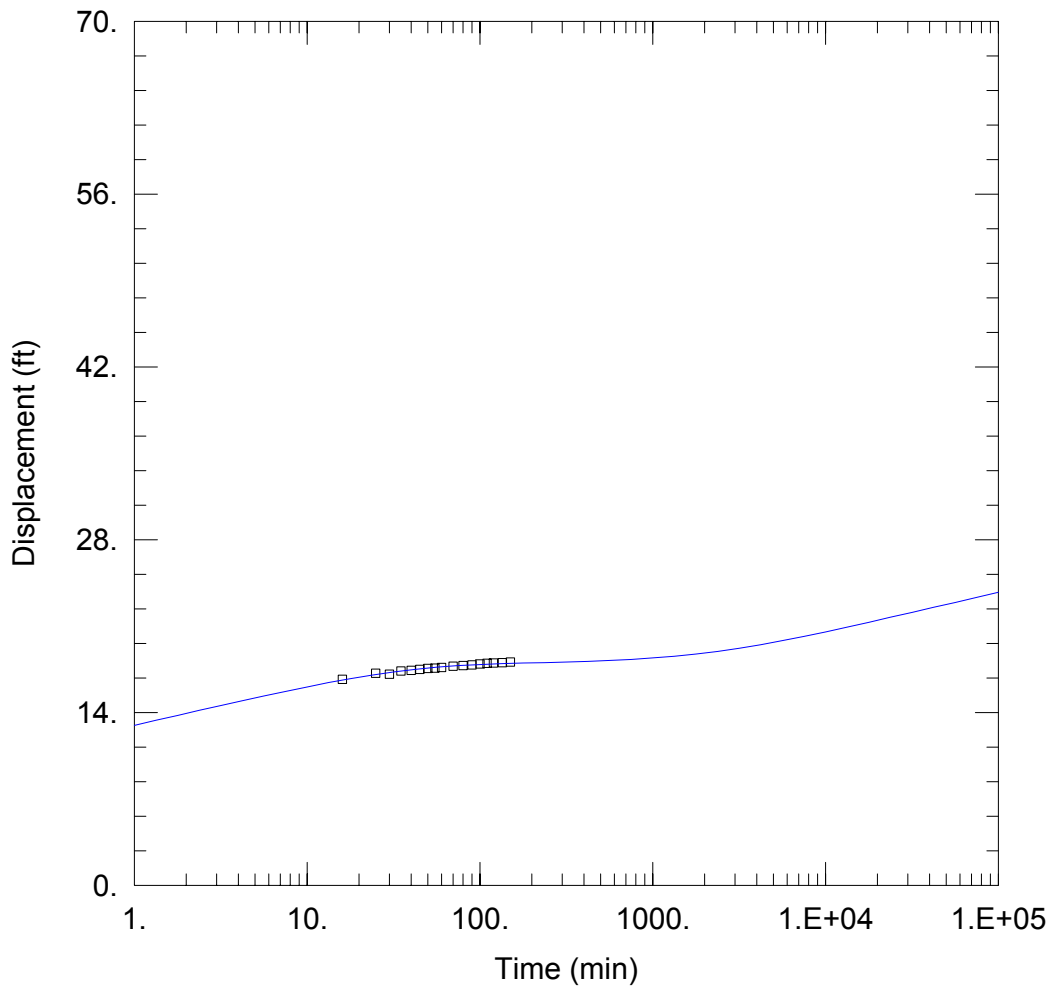
T = 2.21 ft²/min

S = 0.0005653

β = 0.0004798

Kz/Kr = 1.

b = 48. ft



WELL TEST ANALYSIS

Data Set: \...\V016GA_NW.aqt

Date: 11/14/02

Time: 18:46:39

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: V016GA

Test Date: June 1985

AQUIFER DATA

Saturated Thickness: 48. ft

Anisotropy Ratio (K_z/K_r): 1.

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| V016GA | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ V016GA | 0.625 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Neuman-Witherspoon

$T = 1.852 \text{ ft}^2/\text{min}$

$S = 0.001585$

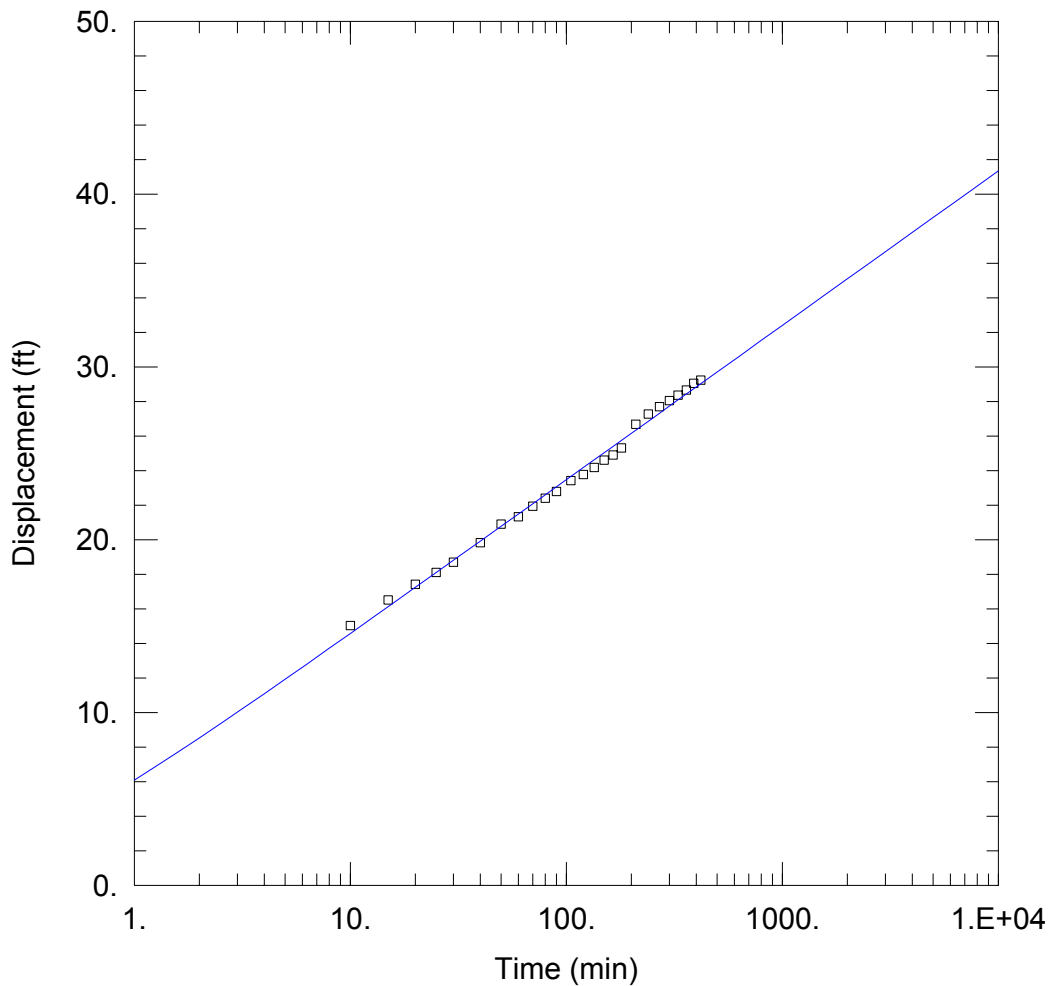
$r/B = 0.002692$

$\beta = 0.0004184$

$T' = 0.1 \text{ ft}^2/\text{min}$

$S' = 0.08913$

Appendix H
AQTESOLV Output for
Well W379 EM (Big Pine)



WELL TEST ANALYSIS

Data Set: \...\379 EM_HJ.aqt

Date: 11/14/02

Time: 15:37:56

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W379 EM

Test Date: August 1986

WELL DATA

Pumping Wells

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W379 EM | 0 | 0 |

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ T627 | 57 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush-Jacob

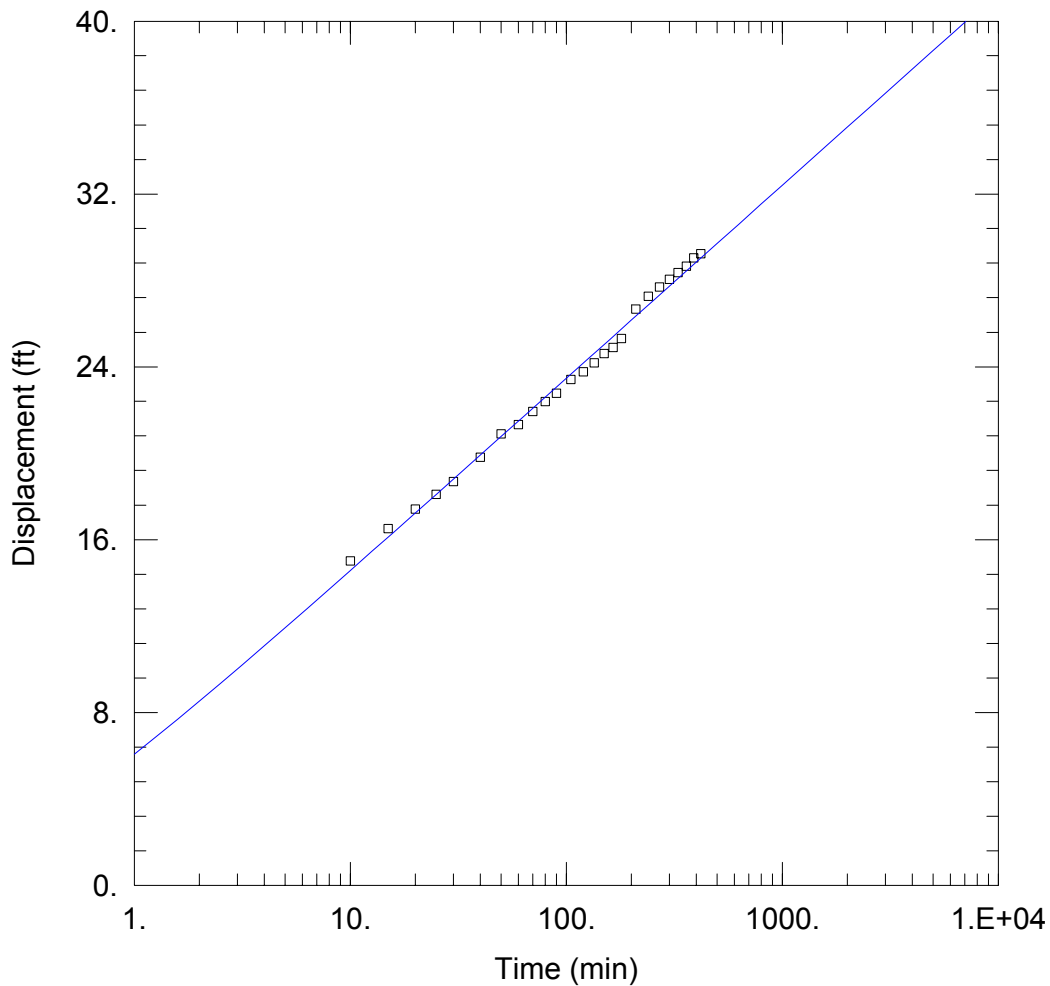
T = 8.385 ft²/min

S = 0.001378

r/B = 1.E-05

Kz/Kr = 1.

b = 200. ft



WELL TEST ANALYSIS

Data Set: \...\379 EM_H.aqt

Date: 11/14/02

Time: 15:44:57

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W379 EM

Test Date: August 1986

WELL DATA

Pumping Wells

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W379 EM | 0 | 0 |

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ T627 | 57 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush

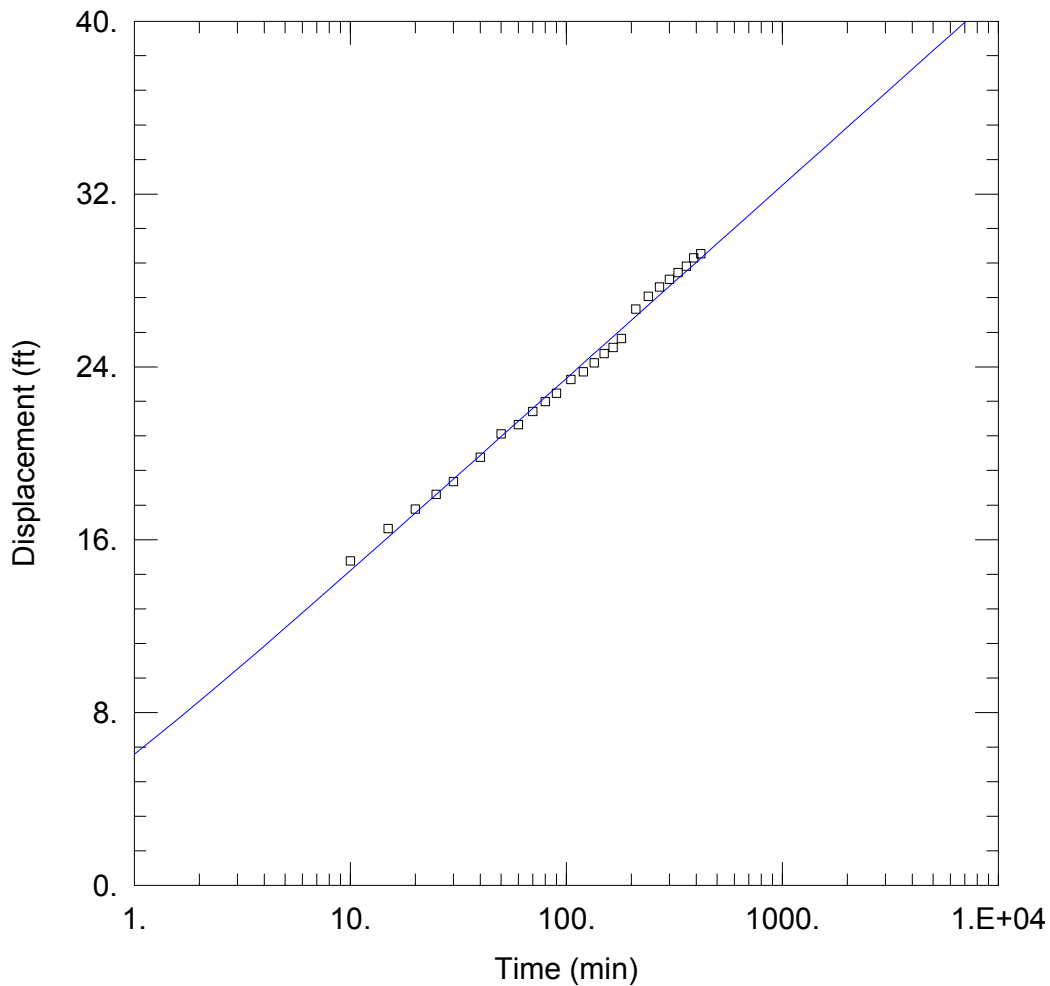
T = 8.378 ft²/min

S = 0.001381

β = 1.E-05

Kz/Kr = 1.

b = 200. ft



WELL TEST ANALYSIS

Data Set: \...\379 EM_NW.aqt

Date: 11/14/02

Time: 15:55:56

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W379 EM

Test Date: August 1986

AQUIFER DATA

Saturated Thickness: 200. ft

Anisotropy Ratio (K_z/K_r): 1.

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W379 EM | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ T627 | 57 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Neuman-Witherspoon

$T = 8.377 \text{ ft}^2/\text{min}$

$S = 0.001382$

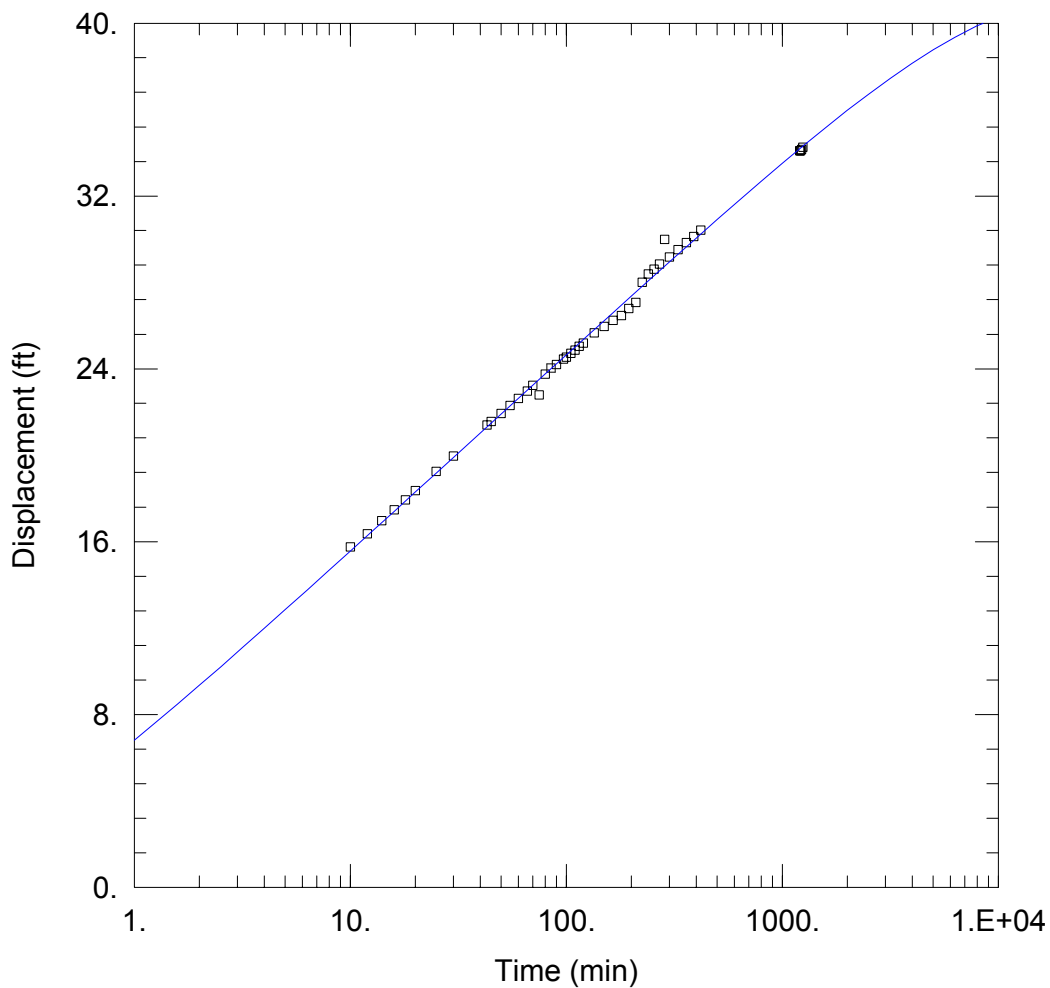
$r/B = 1.E-05$

$\beta = 1.E-05$

$T' = 6430.2 \text{ ft}^2/\text{min}$

$S' = 1.$

Appendix I
AQTESOLV Output for
Well W389 EM (Big Pine)



WELL TEST ANALYSIS

Data Set: \...\389 EM_HJ.aqt

Date: 11/13/02

Time: 19:17:36

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W389 EM

Test Date: April 1987

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W389 EM | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ T736 | 48.5 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush-Jacob

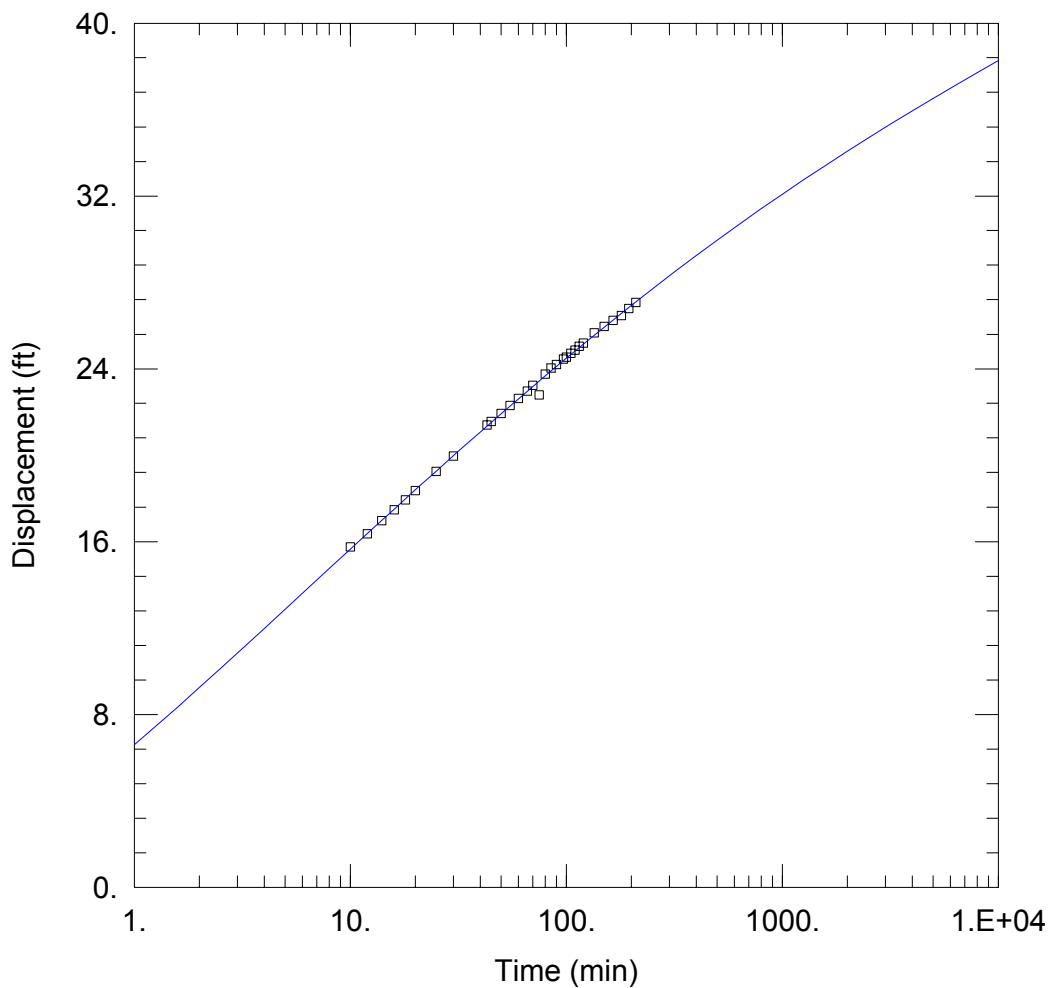
T = 8.376 ft²/min

S = 0.001611

r/B = 0.006052

Kz/Kr = 1.

b = 200. ft



WELL TEST ANALYSIS

Data Set: \...\389 EM_H.aqt

Date: 11/14/02

Time: 10:22:12

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W389 EM

Test Date: April 1987

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W389 EM | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ T736 | 48.5 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush

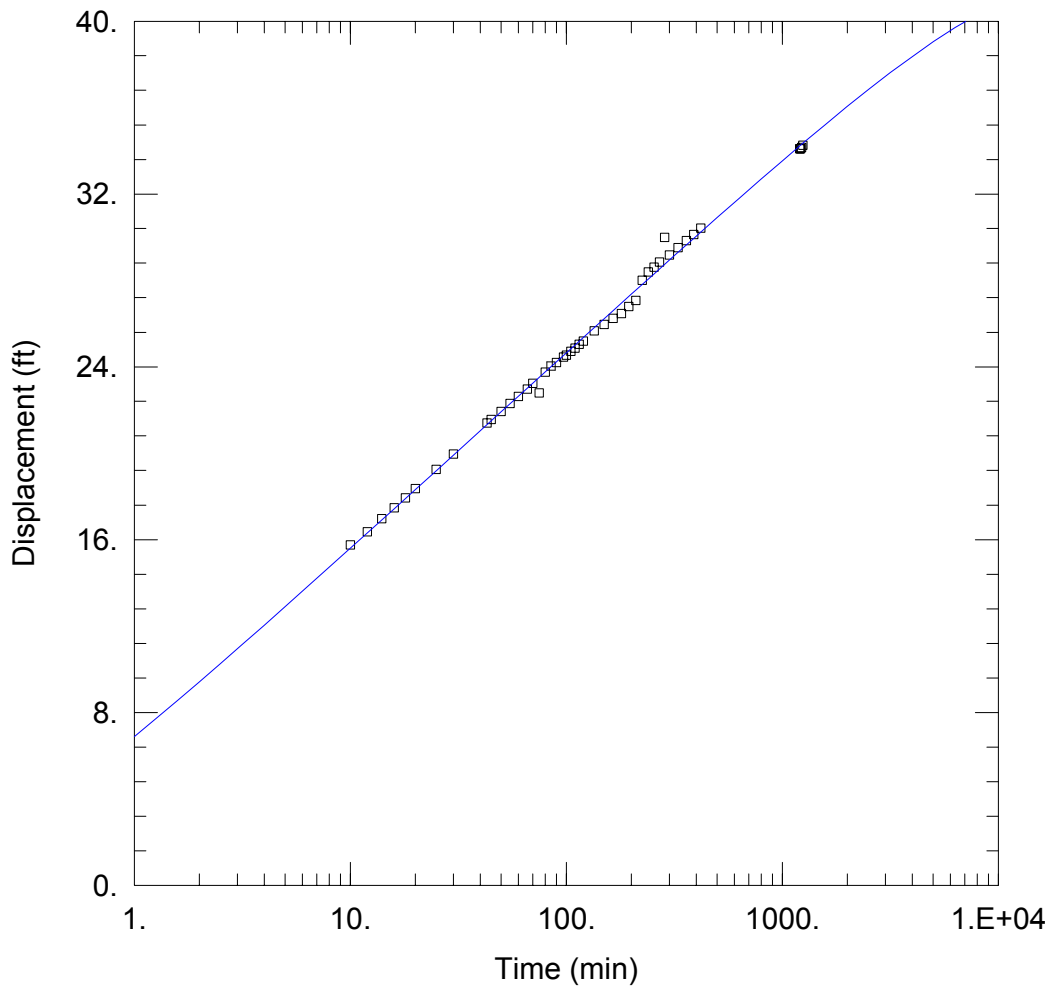
T = 7.694 ft²/min

S = 0.001763

β = 0.007174

Kz/Kr = 1.

b = 200. ft



WELL TEST ANALYSIS

Data Set: \...\389 EM_NW.aqt

Date: 11/14/02

Time: 10:45:30

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W389 EM

Test Date: April 1987

AQUIFER DATA

Saturated Thickness: 200. ft

Anisotropy Ratio (K_z/K_r): 1.

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W389 EM | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ T736 | 48.5 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Neuman-Witherspoon

$T = 8.418 \text{ ft}^2/\text{min}$

$S = 0.001568$

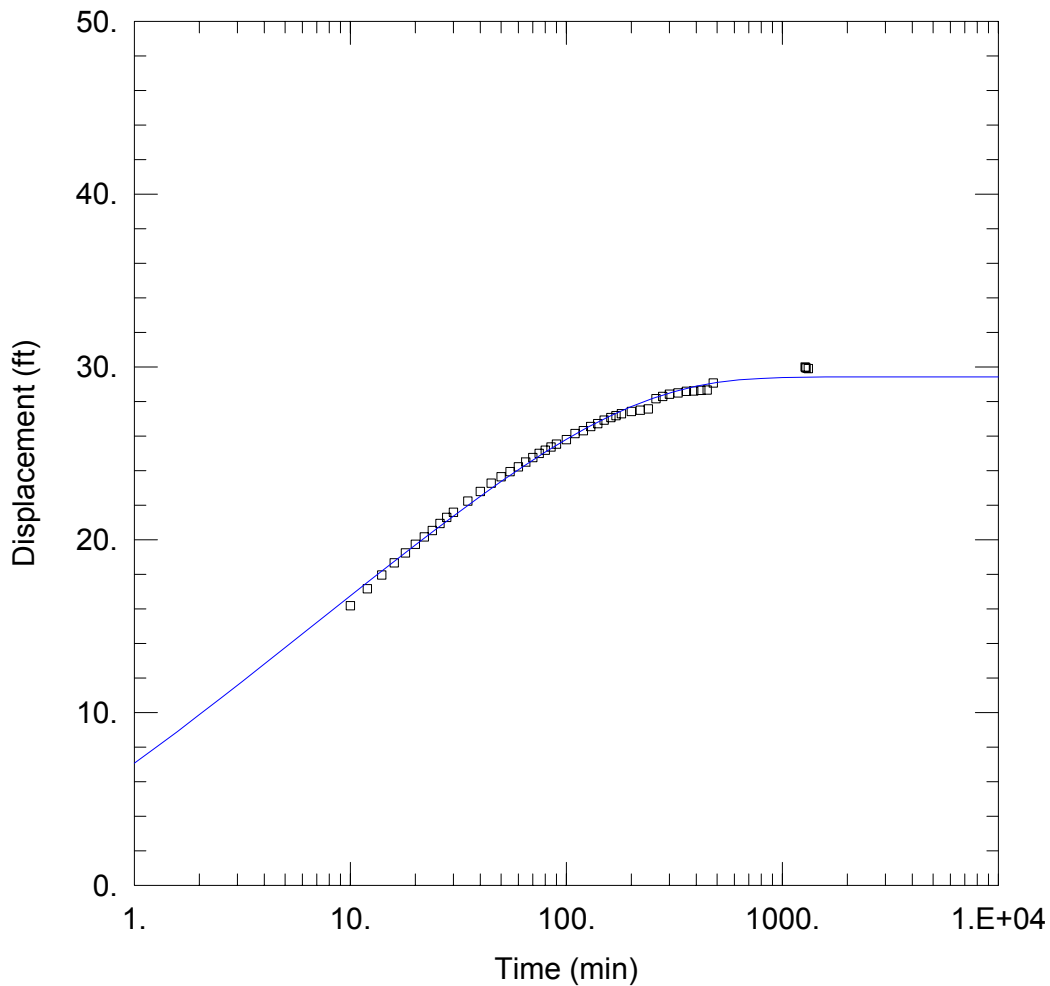
$r/B = 0.005099$

$\beta = 0.0001397$

$T' = 3.626\text{E-}05 \text{ ft}^2/\text{min}$

$S' = 0.818$

Appendix J
AQTESOLV Output for
Well W383 EM (Independence-Oak)



WELL TEST ANALYSIS

Data Set: \...\383 EM_HJ.aqt

Date: 11/13/02

Time: 19:03:05

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W383 EM

Test Date: October 1986

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W383 EM | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ T632 | 81.5 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush-Jacob

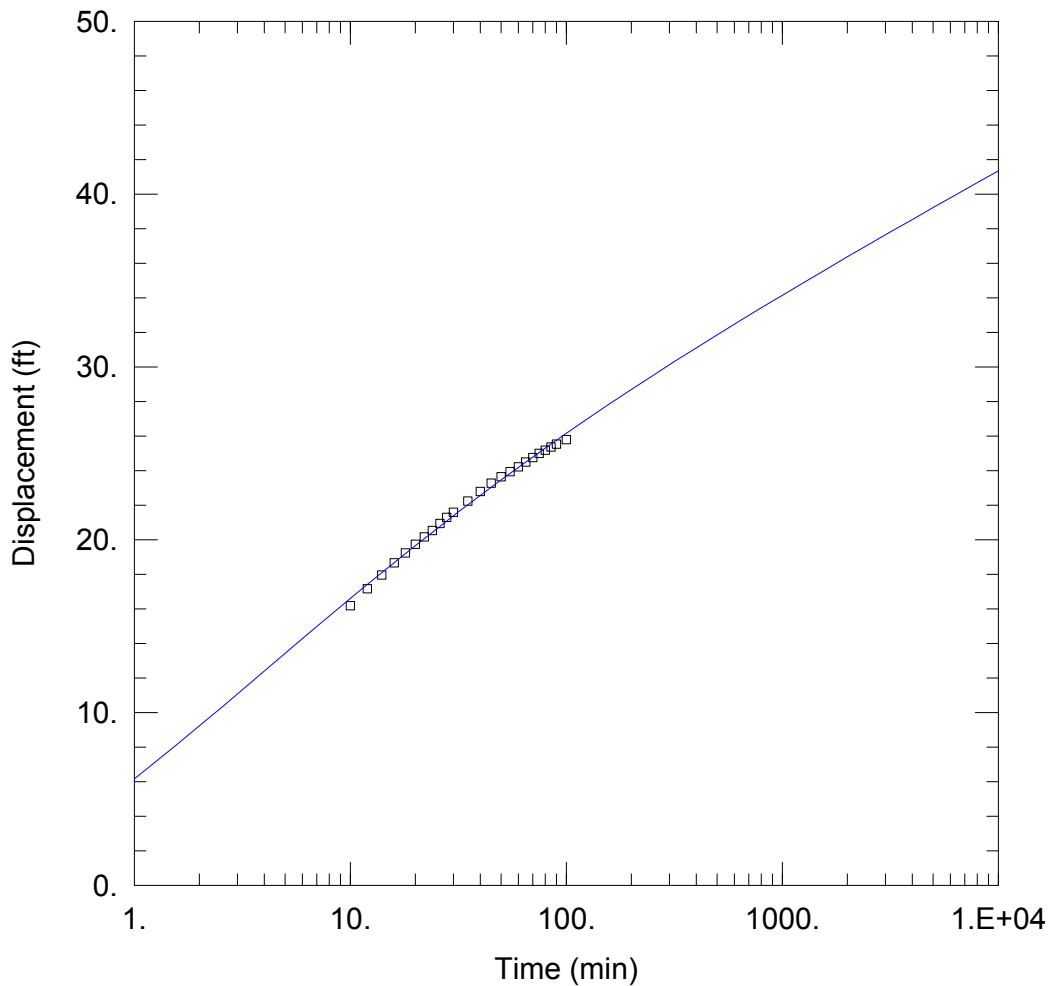
T = 5.232 ft²/min

S = 0.000488

r/B = 0.04637

Kz/Kr = 1.

b = 355. ft



WELL TEST ANALYSIS

Data Set: \...\383 EM_H.aqt

Date: 11/13/02

Time: 19:05:39

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W383 EM

Test Date: October 1986

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W383 EM | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ T632 | 81.5 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush

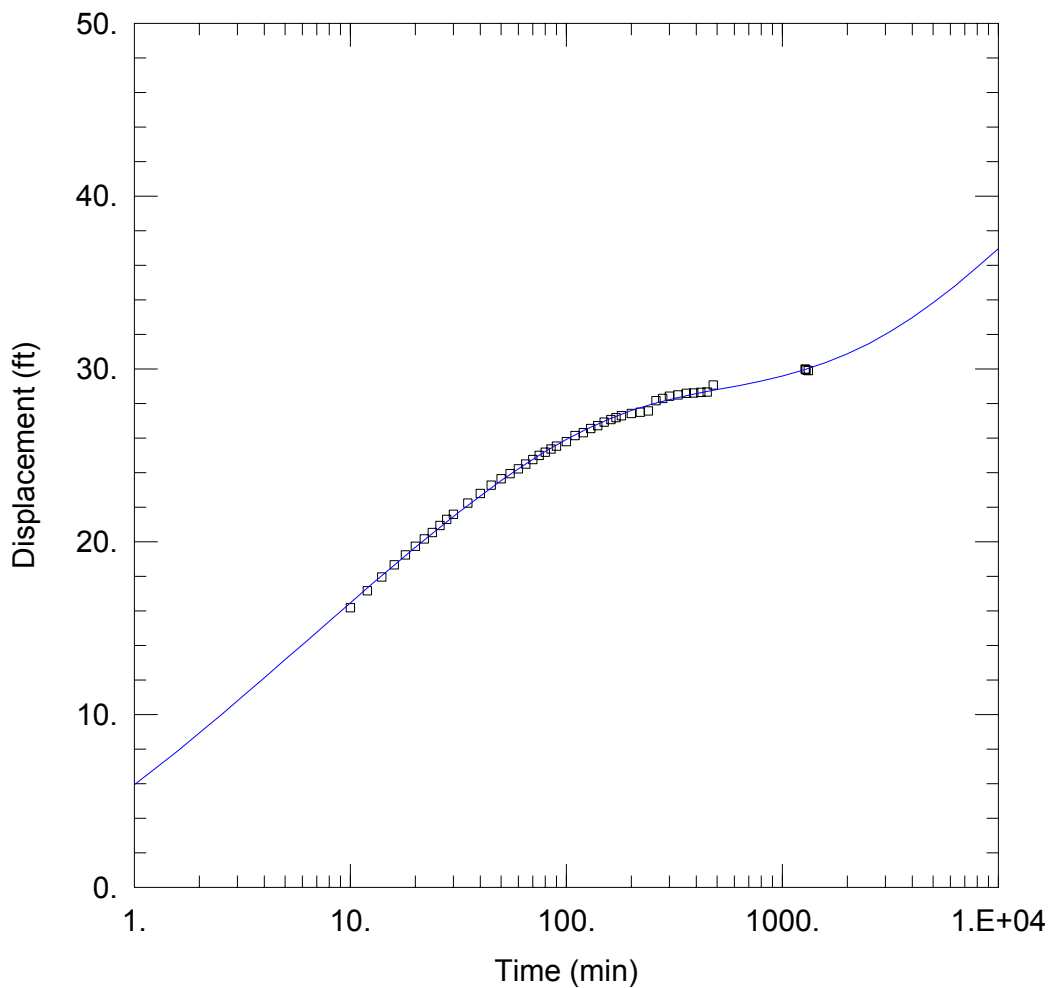
T = 3.997 ft²/min

S = 0.0006054

β = 0.0427

Kz/Kr = 1.

b = 355. ft



WELL TEST ANALYSIS

Data Set: \\...\383 EM_NW.aqt

Date: 11/13/02

Time: 19:08:44

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W383 EM

Test Date: October 1986

AQUIFER DATA

Saturated Thickness: 355 ft

Anisotropy Ratio (K_z/K_r): 1

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W383 EM | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ T632 | 81.5 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Neuman-Witherspoon

$T = 4.641 \text{ ft}^2/\text{min}$

$S = 0.0005891$

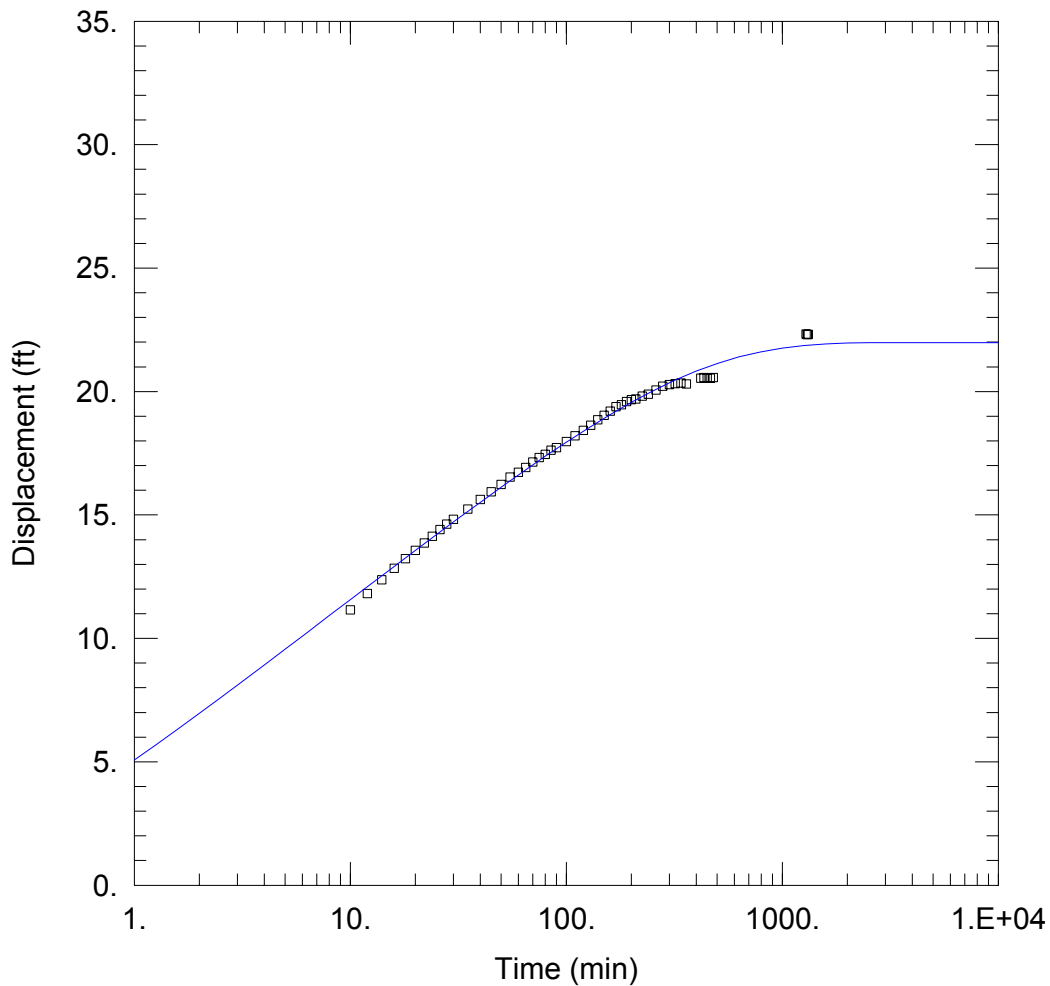
$r/B = 0.06958$

$\beta = 0.004444$

$T' = 1.E-05 \text{ ft}^2/\text{min}$

$S' = 0.01057$

Appendix K
AQTESOLV Output for
Well W384 EM (Independence-Oak)



WELL TEST ANALYSIS

Data Set: \...\384 EM_HJ.aqt

Date: 11/13/02

Time: 18:46:44

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W384 EM

Test Date: September 1986

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W384 EM | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ T633 | 88.5 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush-Jacob

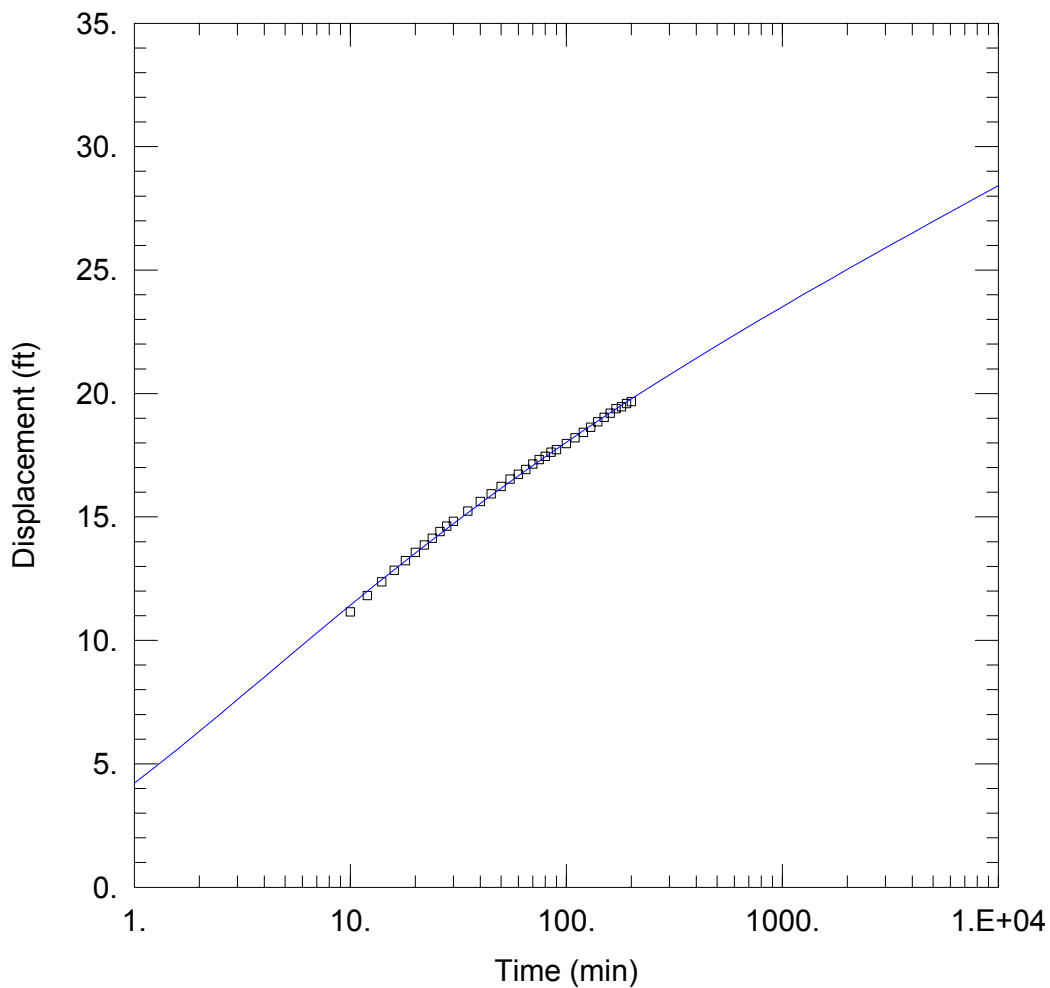
T = 3.401 ft²/min

S = 0.0001972

r/B = 0.02774

Kz/Kr = 1.

b = 340. ft



WELL TEST ANALYSIS

Data Set: \...\384 EM_H.aqt

Date: 11/13/02

Time: 18:50:23

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W384 EM

Test Date: September 1986

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W384 EM | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ T633 | 88.5 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush

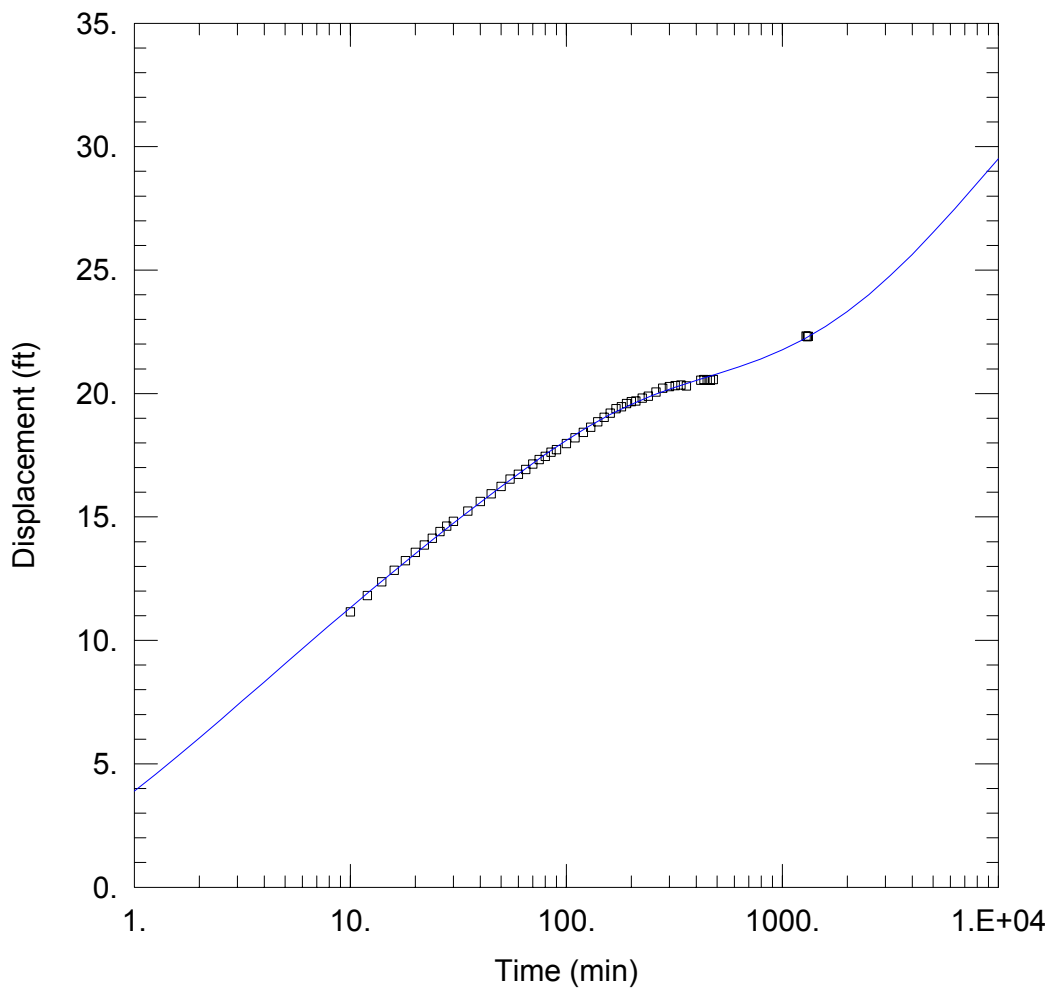
T = 2.526 ft²/min

S = 0.0002785

β = 0.0358

Kz/Kr = 1.

b = 340. ft



WELL TEST ANALYSIS

Data Set: \...\384 EM_NW.aqt

Date: 11/13/02

Time: 18:56:38

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W384 EM

Test Date: September 1986

AQUIFER DATA

Saturated Thickness: 340 ft

Anisotropy Ratio (K_z/K_r): 1

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W384 EM | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ T633 | 88.5 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Neuman-Witherspoon

$T = 2.256 \text{ ft}^2/\text{min}$

$S = 0.0003022$

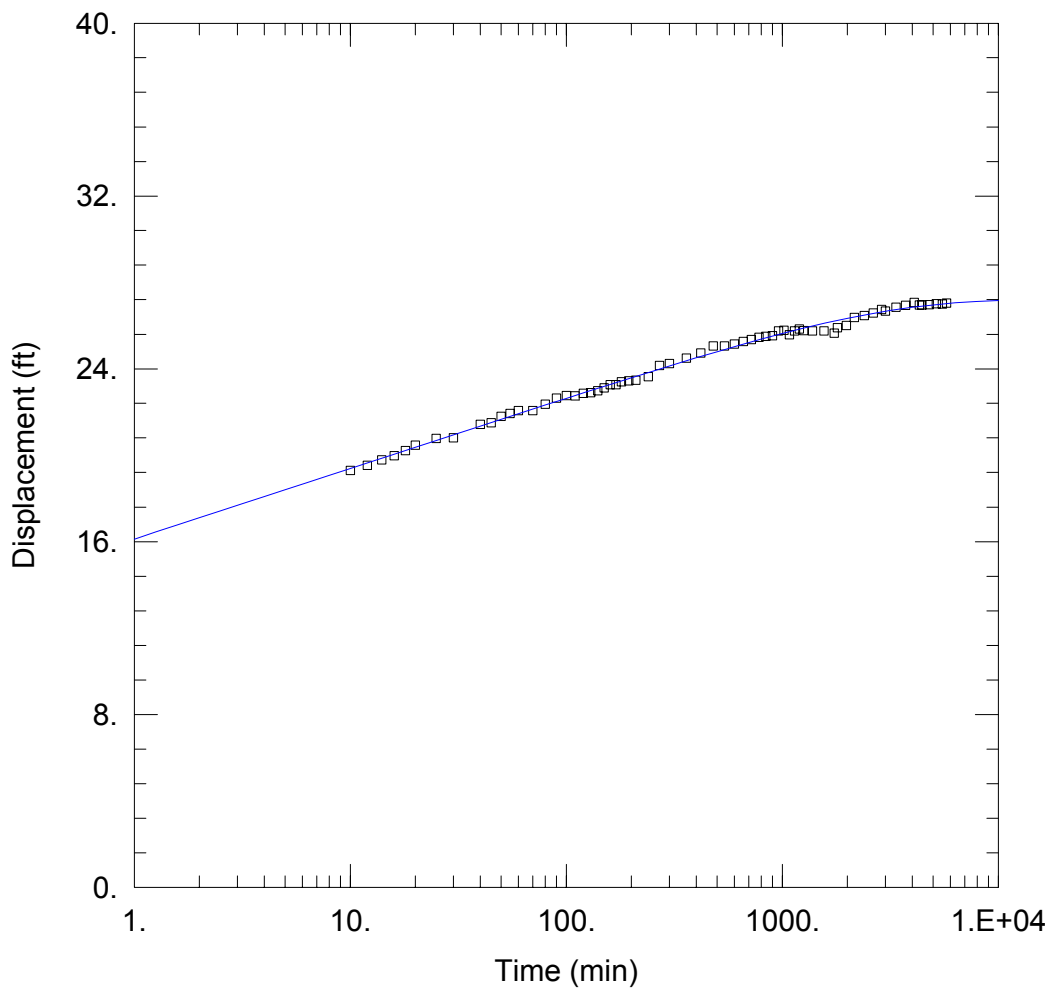
$r/B = 0.1166$

$\beta = 0.05682$

$T' = 1.E-05 \text{ ft}^2/\text{min}$

$S' = 0.007418$

Appendix L
AQTESOLV Output for
Well W395 AQ (Symmes-Shepherd)



WELL TEST ANALYSIS

Data Set: \...\395 AQ_HJ.aqt

Date: 11/14/02

Time: 16:22:39

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W395 AQ

Test Date: January 1989

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W395 AQ | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ W395 AQ | 1.17 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush-Jacob

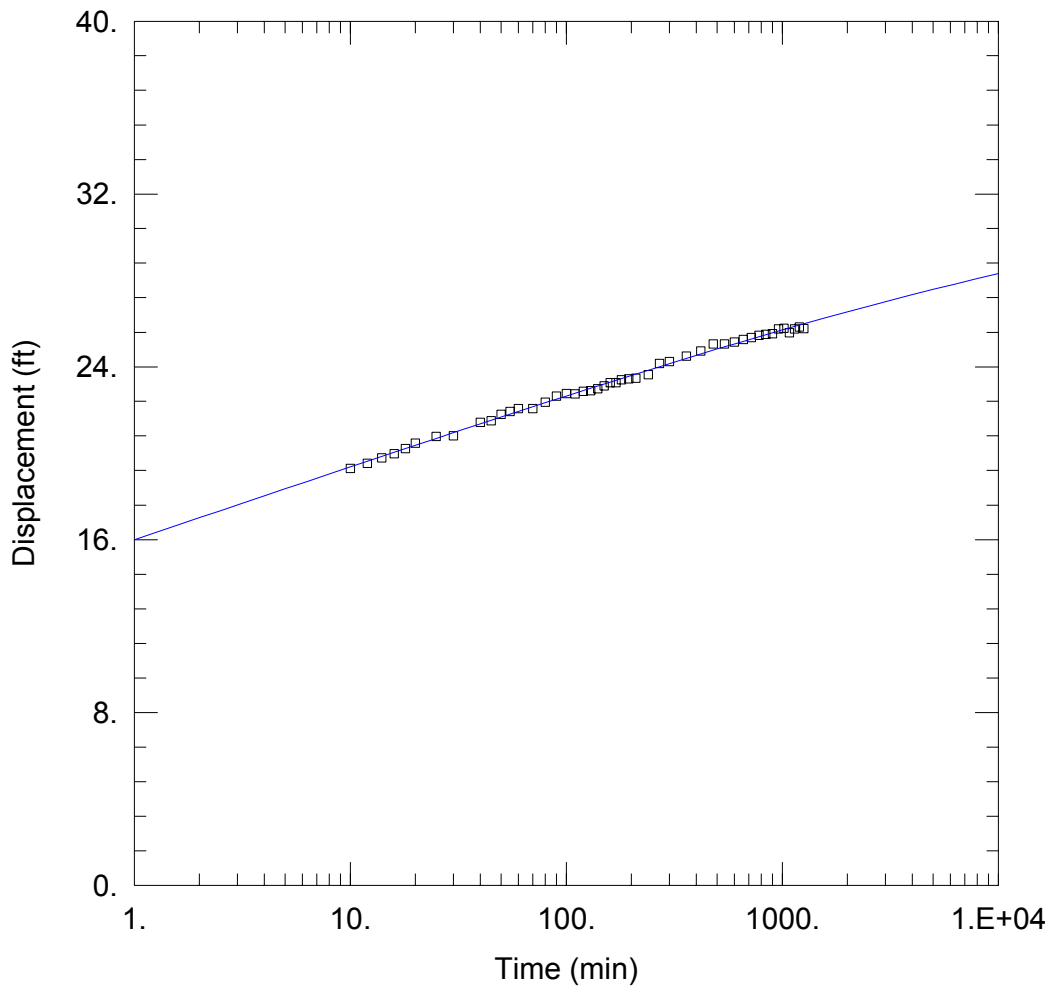
T = 22.82 ft²/min

S = 0.0007491

r/B = 0.0001019

Kz/Kr = 1.

b = 462. ft



WELL TEST ANALYSIS

Data Set: \\...\395 AQ_H.aqt

Date: 11/14/02

Time: 17:15:06

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W395 AQ

Test Date: January 1989

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W395 AQ | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ W395 AQ | 1.17 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Hantush

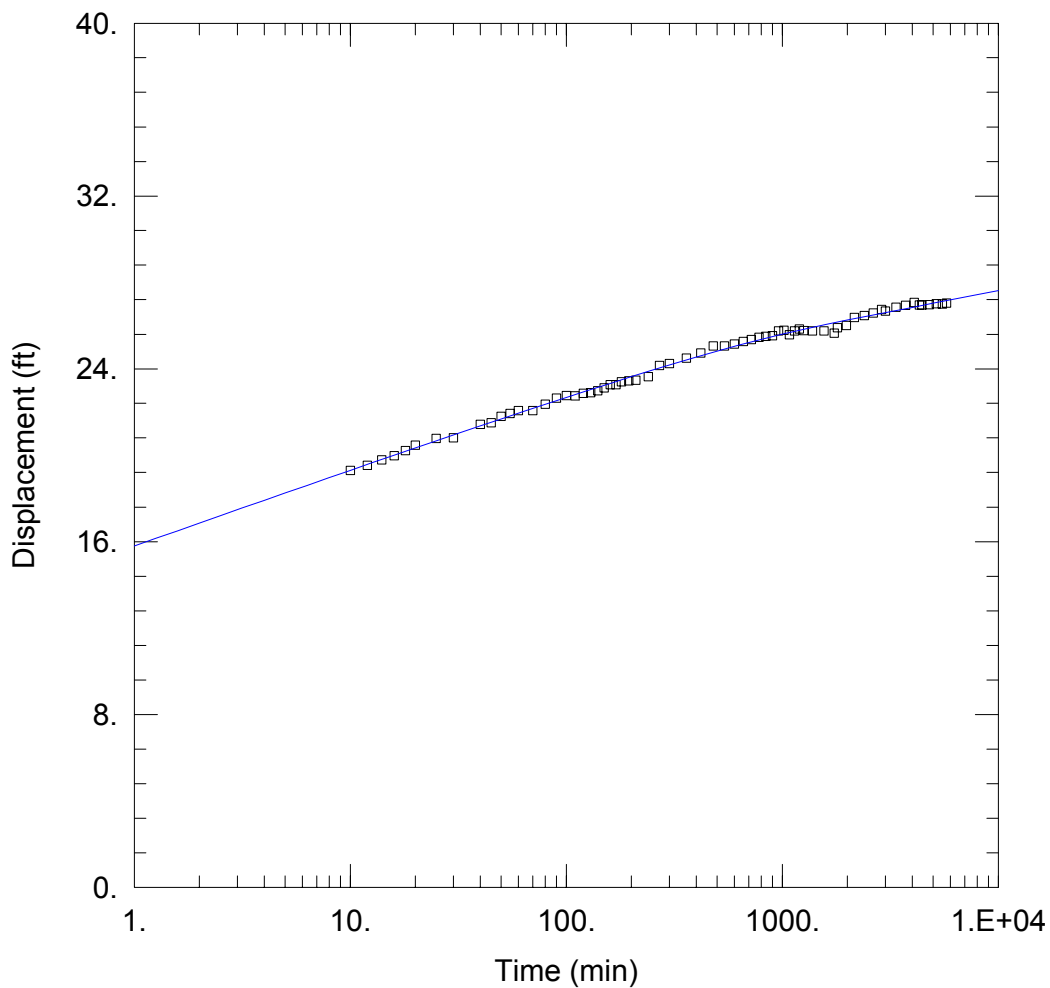
T = 21.88 ft²/min

S = 0.001233

β = 2.687E-05

Kz/Kr = 1.

b = 462. ft



WELL TEST ANALYSIS

Data Set: \...\395 AQ_NW.aqt

Date: 11/14/02

Time: 17:09:25

PROJECT INFORMATION

Company: MWH

Client: LADWP

Test Well: W395 AQ

Test Date: January 1989

AQUIFER DATA

Saturated Thickness: 462. ft

Anisotropy Ratio (Kz/Kr): 1.

WELL DATA

Pumping Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| W395 AQ | 0 | 0 |

Observation Wells

| Well Name | X (ft) | Y (ft) |
|-----------|--------|--------|
| □ W395 AQ | 1.17 | 0 |

SOLUTION

Aquifer Model: Leaky

Solution Method: Neuman-Witherspoon

$T = 20.91 \text{ ft}^2/\text{min}$

$S = 0.001273$

$r/B = 0.0002295$

$\beta = 4.235\text{E-}05$

$T' = 15.7 \text{ ft}^2/\text{min}$

$S' = 0.001318$

Appendix M

Sample Calculations

Appendix M

Sample Calculations

Sample Calculation #1:

**Calculation of the Confining Unit Vertical Hydraulic Conductivity (K_z')
using the Hantush-Jacob Method (1955) for Well W387 EM**

| Known Parameters | Parameter Value for Well W387 EM Analysis |
|---|---|
| Thickness of Confining Unit (b') | 50 feet |
| Radial Distance from the Center of Pumping (r) | 1.17 feet |
| Parameters Yielded by the Hantush-Jacob Method (1955) | Parameter Value for Well W387 EM Analysis |
| Aquifer Transmissivity (T) | 5.298 ft ² /min |
| Aquifer Storativity (S) | 0.004245 |
| Radial distance divided by the leakage factor (r/B) | 0.001337 |

$$\text{By definition, } \left(\frac{r}{B} \right) = \frac{r}{\sqrt{\frac{T \cdot b'}{K_z'}}$$

$$\text{So, } K_z' = \frac{T \cdot b'}{r^2} \cdot \left(\frac{r}{B} \right)^2 = \frac{(5.298 \text{ ft}^2/\text{min}) \cdot (50 \text{ feet})}{(1.17 \text{ feet})^2} \cdot (0.001337)^2 = 0.000346 \text{ ft/min} = 3.73 \text{ gpd/ft}^2$$

Sample Calculation #2:

**Calculation of the Confining Unit Vertical Hydraulic Conductivity/
Storativity Product ($K_z' \cdot S'$) using the Hantush Method (1960) for Well W387 EM**

| Known Parameters | Parameter Value for Well W387 EM Analysis |
|---|---|
| Thickness of Confining Unit (b') | 50 feet |
| Radial Distance from the Center of Pumping (r) | 1.17 feet |
| Parameters Yielded by the Hantush-Jacob Method (1955) | Parameter Value for Well W387 EM Analysis |
| Aquifer Transmissivity (T) | 5.273 ft ² /min |
| Aquifer Storativity (S) | 0.003745 |
| Hantush β | 0.0002972 |

$$\text{By definition, } \beta = \frac{r}{4} \cdot \sqrt{\frac{K_z' \cdot S'}{b' \cdot T \cdot S}}$$

$$\begin{aligned} \text{So, } K_z' \cdot S' &= \frac{16 \cdot \beta^2 \cdot b' \cdot T \cdot S}{r^2} = \frac{16 \cdot (0.0002972)^2 \cdot (50 \text{ feet}) \cdot (5.273 \text{ ft}^2/\text{min}) \cdot (0.003745)}{(1.17 \text{ feet})^2} \\ &= 0.00000102 \text{ ft/min} = 0.0110 \text{ gpd/ft}^2 \end{aligned}$$

Note: The storativity of the confining unit may be estimated by dividing the confining unit vertical hydraulic conductivity/storativity product calculated with the Hantush Method by one of the confining unit vertical hydraulic conductivity estimates generated with either the Hantush-Jacob Method or the Neuman-Witherspoon Method. However, because the resultant storativity value would be highly uncertain due to the combination of two separate analysis methods, and because obtaining the confining unit storativity was not the ultimate objective of this analysis, this calculation was not performed.

Sample Calculation #3:

**Calculation of the Confining Unit Vertical Hydraulic Conductivity (K_z')
using the Neuman-Witherspoon Method (1969) for Well W387 EM**

| Known Parameters | Parameter Value for Well W387 EM Analysis |
|--|--|
| Thickness of Confining Unit (b') | 50 feet |
| Radial Distance from the Center of Pumping (r) | 1.17 feet |
| Parameters Yielded by the Hantush-Jacob Method (1955) | Parameter Value for Well W387 EM Analysis |
| Confined Aquifer Transmissivity (T) | 4.976 ft ² /min |
| Confined Aquifer Storativity (S) | 0.002734 |
| Radial distance divided by the leakage factor (r/B) | 0.001344 |
| Neuman-Witherspoon β | 0.0003301 |
| Unconfined Aquifer Transmissivity (T^U) | 0.00001243 |
| Unconfined Aquifer Storativity (S^U) | 1.0 |

$$\text{By definition, } \left(\frac{r}{B} \right) = \frac{r}{\sqrt{\frac{T \cdot b'}{K_z'}}$$

$$\text{So, } K_z' = \frac{T \cdot b'}{r^2} \cdot \left(\frac{r}{B} \right)^2 = \frac{(4.976 \text{ ft}^2/\text{min}) \cdot (50 \text{ feet})}{(1.17 \text{ feet})^2} \cdot (0.001344)^2 = 0.000328 \text{ ft/min} = 3.54 \text{ gpd/ft}^2$$

Note, confining unit vertical hydraulic conductivity can also be obtained from the Neuman-Witherspoon β value. However, in order to perform this calculation, both the confined aquifer vertical hydraulic conductivity and the confining unit storativity must be known. See below:

$$\text{By definition, } \beta = \frac{r}{4b} \sqrt{\frac{K_z' \cdot S'}{K_z \cdot S}} \rightarrow K_z' = \frac{16b^2 \cdot \beta^2}{r^2} \cdot \frac{K_z \cdot S}{S'}$$

Where: b = Confined Aquifer Thickness

K_z = Confined Aquifer Vertical Hydraulic Conductivity

S' = Confining Unit Storativity

Due to these limitations, calculation of the confining unit vertical hydraulic conductivity was performed only using the calculated r/B value for the purposes of this technical memorandum.